

Storm Drainage Systems

Purpose: Convey rainwater from building roof drains, area drains, subsoil drains, and foundation drains to a point of discharge, subsurface dispersal, or reuse.

Authority Having Jurisdiction (AHJ) Regulates the Discharge Point:

- Municipal Sewer
- Combined Sewer
- Ground Surface
- Subsurface

Codes, Standards, and Regulations

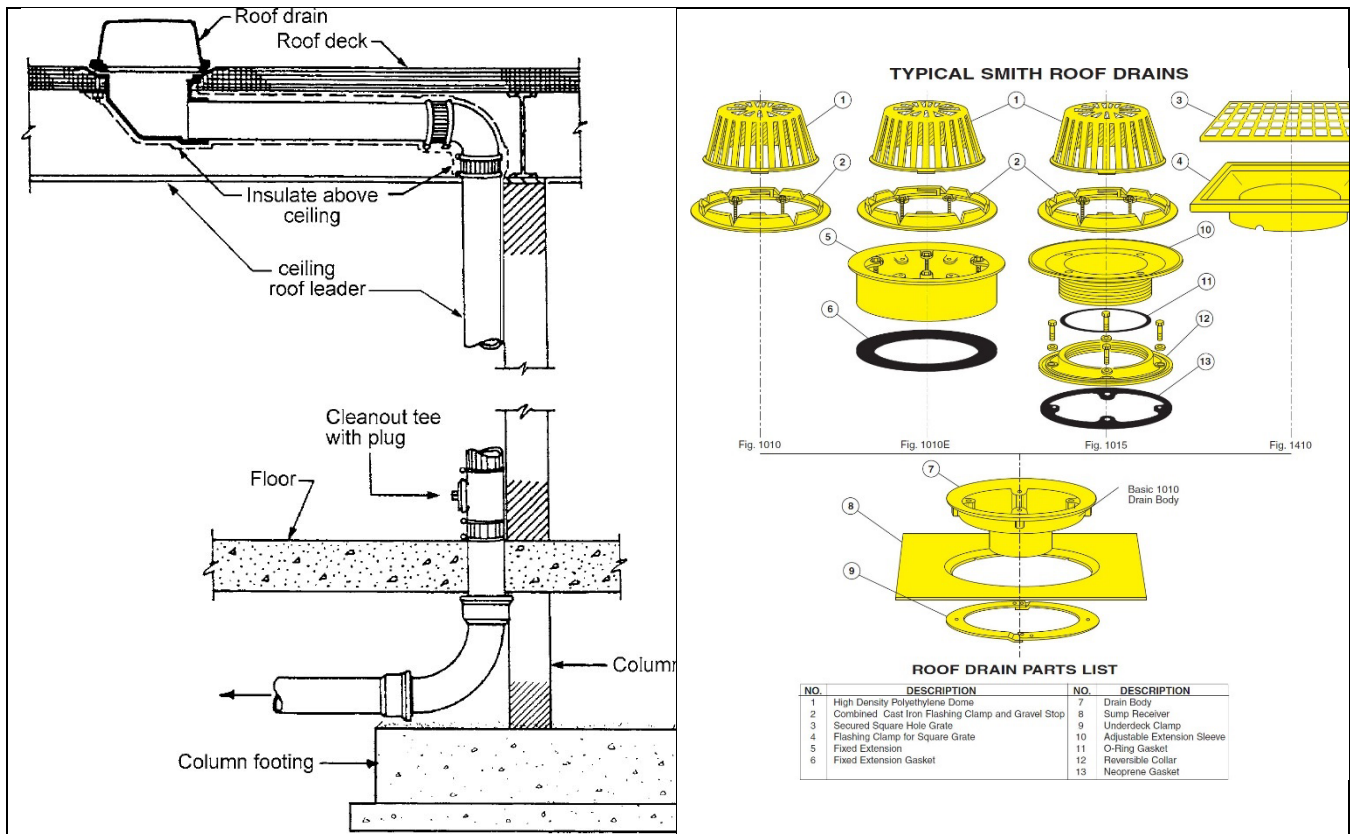
Environmental protection laws establish the requirements for groundwater and surface-water protection. Environmental agencies regulate watersheds based on specific locations. Local codes, ordinances and laws are the primary requirements used and must be obtained from the local AHJ. Clean Water Act (CWA)
 U.S. Environmental Protection Agency (EPA)
 National Pollutant Discharge Elimination System (NPDES)

Materials

Stormwater system components include piping, fittings, bedding, backfill, supports and hangers, fixtures, and treatment devices. Must be approved by the AHJ

Interior Building Drainage System

Based on local code requirements. All designs must meet or exceed the local code requirements.



Typical Roof Drain and Roof Leader

Sizing Roof Drainage System

The sizing of the roof drainage system must consider all components: the rain storm, the flow of rainwater on the roof surface, the ability of the roof drain to move the water from the roof to the piping, the vertical piping, the horizontal piping and the connection to the point of discharge.

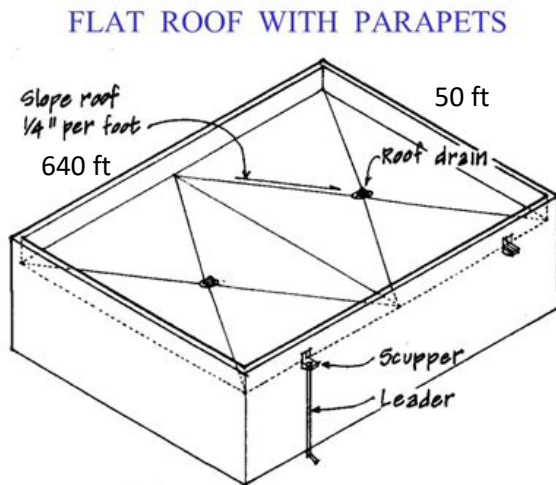
Four Steps to Sizing Roof Drains on a Building

1. Determine 100-year, 1-hour rainfall rate used for location of building
2. Select number of roof drains and calculate the roof area sloped to each drain
3. Size horizontal storm drainage piping
4. Size vertical storm drainage piping

2016 California Plumbing Code - Appendix D Sizing Storm Water Drainage Systems provides maximum rates of rainfall for U.S. various cities based on inches per hour that may fall during a storm of one-hour duration and a 100-year return period. In other words, a storm so severe and with so much rain it will only occur once in 100 years. The goal is to plan for the absolute worst and be able to easily accommodate the typical rainfall amounts.

Example 1.

The building shown is in Redding, California. Determine the minimum size drain, horizontal drainage pipe and leader that can be used for the drainage design shown.



1. Appendix D – Table D 101.1

**TABLE D 101.1
MAXIMUM RATES OF RAINFALL FOR VARIOUS CITIES* (continued)**

| STATES AND CITIES | STORM DRAINAGE 60-MINUTE DURATION, 100-YEAR RETURN | |
|-------------------|--|------------------------------------|
| | inches per hour | gallons per minute per square foot |
| CALIFORNIA | – | – |
| Eureka | 1.5 | 0.016 |
| Lake Tahoe | 1.3 | 0.014 |
| Los Angeles | 2.0 | 0.021 |
| Lucerne Valley | 2.5 | 0.026 |
| Needles | 1.5 | 0.016 |
| Palmdale | 3.0 | 0.031 |
| Redding | 1.5 | 0.016 |
| San Diego | 1.5 | 0.016 |
| San Francisco | 1.5 | 0.016 |
| San Luis Obispo | 1.5 | 0.016 |

2. Two Drains, 1/4" per 12" slope
Size the Drain: For 1.5 inches per hour use Note 2.

**TABLE 1101.12
SIZING ROOF DRAINS, LEADERS, AND VERTICAL RAINWATER PIPING^{2, 3}**

| SIZE OF DRAIN, LEADER, OR PIPE | FLOW | MAXIMUM ALLOWABLE HORIZONTAL PROJECTED ROOF AREAS AT VARIOUS RAINFALL RATES (square feet) | | | | | | | | | | | | |
|--------------------------------|------|---|------------------|----------|----------|----------|----------|----------|----------|----------|----------|----------|-----------|-----------|
| | | inches | gpm ¹ | 1 (in/h) | 2 (in/h) | 3 (in/h) | 4 (in/h) | 5 (in/h) | 6 (in/h) | 7 (in/h) | 8 (in/h) | 9 (in/h) | 10 (in/h) | 11 (in/h) |
| 2 | 30 | | 2880 | 1440 | 960 | 720 | 575 | 480 | 410 | 360 | 320 | 290 | 260 | 240 |
| 3 | 92 | | 8800 | 4400 | 2930 | 2200 | 1760 | 1470 | 1260 | 1100 | 980 | 880 | 800 | 730 |
| 4 | 192 | | 18 400 | 9200 | 6130 | 4600 | 3680 | 3070 | 2630 | 2300 | 2045 | 1840 | 1675 | 1530 |
| 5 | 360 | | 34 600 | 17 300 | 11 530 | 8650 | 6920 | 5765 | 4945 | 4325 | 3845 | 3460 | 3145 | 2880 |
| 6 | 563 | | 54 000 | 27 000 | 17 995 | 13 500 | 10 800 | 9000 | 7715 | 6750 | 6000 | 5400 | 4910 | 4500 |
| 8 | 1208 | | 116 000 | 58 000 | 38 660 | 29 000 | 23 200 | 19 315 | 16 570 | 14 500 | 12 890 | 11 600 | 10 545 | 9600 |

For SI units: 1 inch = 25 mm, 1 gallon per minute = 0.06 L/s, 1 inch per hour = 25.4 mm/h, 1 square foot = 0.0929 m²

Notes:

- ¹ Maximum discharge capacity, gpm (L/s) with approximately 1¾ inch (44 mm) head of water at the drain.
- ² For rainfall rates other than those listed, determine the allowable roof area by dividing the area given in the 1 inch per hour (25.4 mm/h) column by the desired rainfall rate.
- ³ Vertical piping shall be round, square, or rectangular. Square pipe shall be sized to enclose its equivalent roundpipe. Rectangular pipe shall have not less than the same cross-sectional area as its equivalent round pipe, except that the ratio of its side dimensions shall not exceed 3 to 1.

3. Size the Horizontal Drainage Pipe

**TABLE 1101.8
SIZING OF HORIZONTAL RAINWATER PIPING^{1, 2}**

| SIZE OF PIPE | FLOW (½ inch per foot slope) | MAXIMUM ALLOWABLE HORIZONTAL PROJECTED ROOF AREAS AT VARIOUS RAINFALL RATES (square feet) | | | | | | |
|--------------|---------------------------------|---|---------|----------|----------|----------|----------|----------|
| | | inches | gpm | 1 (in/h) | 2 (in/h) | 3 (in/h) | 4 (in/h) | 5 (in/h) |
| 3 | 34 | | 3288 | 1644 | 1096 | 822 | 657 | 548 |
| 4 | 78 | | 7520 | 3760 | 2506 | 1880 | 1504 | 1253 |
| 5 | 139 | | 13 360 | 6680 | 4453 | 3340 | 2672 | 2227 |
| 6 | 222 | | 21 400 | 10 700 | 7133 | 5350 | 4280 | 3566 |
| 8 | 478 | | 46 000 | 23 000 | 15 330 | 11 500 | 9200 | 7670 |
| 10 | 860 | | 82 800 | 41 400 | 27 600 | 20 700 | 16 580 | 13 800 |
| 12 | 1384 | | 133 200 | 66 600 | 44 400 | 33 300 | 26 650 | 22 200 |
| 15 | 2473 | | 238 000 | 119 000 | 79 333 | 59 500 | 47 600 | 39 650 |

| SIZE OF PIPE | FLOW (¼ inch per foot slope) | MAXIMUM ALLOWABLE HORIZONTAL PROJECTED ROOF AREAS AT VARIOUS RAINFALL RATES (square feet) | | | | | | |
|--------------|---------------------------------|---|---------|----------|----------|----------|----------|----------|
| | | inches | gpm | 1 (in/h) | 2 (in/h) | 3 (in/h) | 4 (in/h) | 5 (in/h) |
| 3 | 48 | | 4640 | 2320 | 1546 | 1160 | 928 | 773 |
| 4 | 110 | | 10 600 | 5300 | 3533 | 2650 | 2120 | 1766 |
| 5 | 196 | | 18 880 | 9440 | 6293 | 4720 | 3776 | 3146 |
| 6 | 314 | | 30 200 | 15 100 | 10 066 | 7550 | 6040 | 5033 |
| 8 | 677 | | 65 200 | 32 600 | 21 733 | 16 300 | 13 040 | 10 866 |
| 10 | 1214 | | 116 800 | 58 400 | 38 950 | 29 200 | 23 350 | 19 450 |
| 12 | 1953 | | 188 000 | 94 000 | 62 600 | 47 000 | 37 600 | 31 350 |
| 15 | 3491 | | 336 000 | 168 000 | 112 000 | 84 000 | 67 250 | 56 000 |

4. Size the Vertical Drainage Pipe

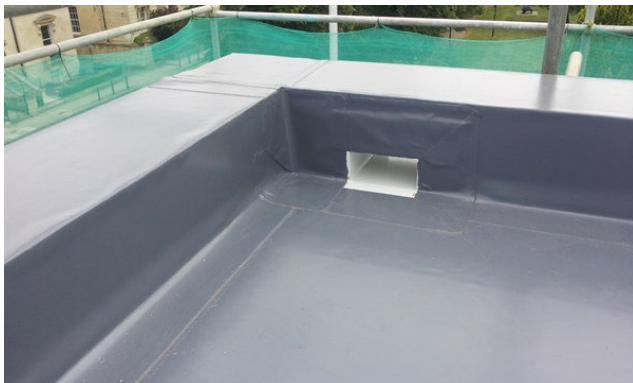
Table 1101.12

What are the minimum requirements for the scupper?

1101.12.2 Secondary Drainage. Secondary (emergency) roof drainage shall be provided by one of the methods specified in Section 1101.12.2.1 or Section 1101.12.2.2. <<

1101.12.2.1 Roof Scuppers or Open Side. <<

Secondary roof drainage shall be provided by an open-sided roof or scuppers where the roof perimeter construction extends above the roof in such a manner that water will be entrapped. An open-sided roof or scuppers shall be sized to prevent the depth of ponding water from exceeding that for which the roof was designed as determined by Section 1101.12.1. Scupper openings shall be not less than 4 inches (102 mm) high and have a width equal to the circumference of the roof drain required for the area served, sized in accordance with Table 1101.12.



Example 2.

Size a horizontal roof drain serving 14,000 square feet of roof area on a building in Minneapolis, MN.

1. Appendix D – 2016 California Plumbing Code Table D101.1 (60 min, 100 yr)

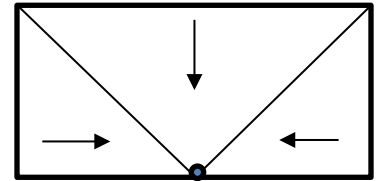
Minneapolis MN – Always Check Code Amendments

Minnesota Plumbing Code 1101.11 Roof Drainage

1101.11.1 Primary Roof Drainage

“Shall be sized on a basis of a rate of rainfall at a minimum 4 inches per hour.”

2. Number of drains and area sloped to each drain.



3. Size Horizontal Drainage Pipe

**TABLE 1101.7
SIZING OF HORIZONTAL RAINWATER PIPING^{1, 2}**

| SIZE OF PIPE inches | FLOW (1/8 inch per foot slope) gpm | MAXIMUM ALLOWABLE HORIZONTAL PROJECTED ROOF AREAS AT VARIOUS RAINFALL RATES (square feet) | | | | | |
|------------------------|---|---|----------|----------|----------|----------|----------|
| | | 1 (in/h) | 2 (in/h) | 3 (in/h) | 4 (in/h) | 5 (in/h) | 6 (in/h) |
| 3 | 34 | 3288 | 1644 | 1096 | 822 | 657 | 548 |
| 4 | 78 | 7520 | 3760 | 2506 | 1880 | 1504 | 1253 |
| 5 | 139 | 13 360 | 6680 | 4453 | 3340 | 2672 | 2227 |
| 6 | 222 | 21 400 | 10 700 | 7133 | 5350 | 4280 | 3566 |
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| 10 | 860 | 82 800 | 41 400 | 27 600 | 20 700 | 16 580 | 13 800 |
| 12 | 1384 | 133 200 | 66 600 | 44 400 | 33 300 | 26 650 | 22 200 |
| 15 | 2473 | 238 000 | 119 000 | 79 333 | 59 500 | 47 600 | 39 650 |

4. Size the Vertical Drainage Pipe

**TABLE 1101.11
SIZING ROOF DRAINS, LEADERS, AND VERTICAL RAINWATER PIPING^{2, 3}**

| SIZE OF DRAIN, LEADER, OR PIPE inches | FLOW gpm ¹ | MAXIMUM ALLOWABLE HORIZONTAL PROJECTED ROOF AREAS AT VARIOUS RAINFALL RATES (square feet) | | | | | | | | | | | |
|---|--------------------------|---|----------|----------|----------|----------|----------|----------|----------|----------|-----------|-----------|-----------|
| | | 1 (in/h) | 2 (in/h) | 3 (in/h) | 4 (in/h) | 5 (in/h) | 6 (in/h) | 7 (in/h) | 8 (in/h) | 9 (in/h) | 10 (in/h) | 11 (in/h) | 12 (in/h) |
| 2 | 30 | 2880 | 1440 | 960 | 720 | 575 | 480 | 410 | 360 | 320 | 290 | 260 | 240 |
| 3 | 92 | 8800 | 4400 | 2930 | 2200 | 1760 | 1470 | 1260 | 1100 | 980 | 880 | 800 | 730 |
| 4 | 192 | 18 400 | 9200 | 6130 | 4600 | 3680 | 3070 | 2630 | 2300 | 2045 | 1840 | 1675 | 1530 |
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| 6 | 563 | 54 000 | 27 000 | 17 995 | 13 500 | 10 800 | 9000 | 7715 | 6750 | 6000 | 5400 | 4910 | 4500 |
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For SI units: 1 inch = 25 mm, 1 gallon per minute = 0.06 L/s, 1 inch per hour = 25.4 mm/h, 1 square foot = 0.0929 m²

Notes:

¹ Maximum discharge capacity, gpm (L/s) with approximately 1 3/4 inch (44 mm) head of water at the drain.

² For rainfall rates other than those listed, determine the allowable roof area by dividing the area given in the 1 inch per hour (25.4 mm/h) column by the desired rainfall rate.

³ Vertical piping shall be round, square, or rectangular. Square pipe shall be sized to enclose its equivalent roundpipe. Rectangular pipe shall have not less than the same cross-sectional area as its equivalent round pipe, except that the ratio of its side dimensions shall not exceed 3 to 1.

1106.3 Reduction in Size Prohibited. Except for siphonic roof drainage systems, storm drain piping shall not reduce in size in the direction of flow, including changes in direction from horizontal to vertical.

Designing a Stormwater Harvesting System

General rule of thumb when sizing a cistern to capture rainfall is that 1-inch rainfall from a 1000 square foot roof will generate approximately 600 gallons of runoff.



Cistern Volume

Equation $V = A^2 \times i \times c \times 7.5 \text{ gal/ft}^3$

V = volume of rain barrel (gallons)

A² = surface area roof (square feet)

i = rainfall (1 inch = 0.08 ft)

c = coefficient of runoff (0.9 for impervious areas)

7.5 = conversion factor (gallons per cubic foot)

Example 3

Calculate the volume of runoff that can be captured from a 3500 square foot roof in Chico, CA.

Historic Average

[Home](#) » [Local](#) » [Historic Averages](#)

Chico, California

[Weather Report](#) · [Interactive Map](#) · [Extended Forecast](#) · [Hourly Forecast](#) · [Past Observations](#) · [Historic Averages](#)

| Monthly Averages & Records - °F °C | | | | | | |
|--------------------------------------|-------------|--------------|------------|-------------|-----------------------|--------------|
| Date | Average Low | Average High | Record Low | Record High | Average Precipitation | Average Snow |
| January | 35° | 54° | 12° (1937) | 77° (1920) | 5.17" | 0.1" |
| February | 39° | 60° | 16° (1989) | 82° (1920) | 4.5" | 0" |
| March | 42° | 65° | 23° (1971) | 93° (1914) | 4.32" | 0" |
| April | 45° | 72° | 27° (1922) | 98° (1926) | 1.59" | 0" |
| May | 51° | 80° | 30° (1984) | 108° (1910) | 0.91" | 0" |
| June | 57° | 88° | 38° (1986) | 115° (1961) | 0.47" | 0" |
| July | 61° | 93° | 40° (1981) | 117° (1972) | 0.05" | 0" |
| August | 59° | 92° | 38° (1978) | 116° (1914) | 0.16" | 0" |
| September | 55° | 88° | 35° (1986) | 114° (1988) | 0.59" | 0" |
| October | 47° | 79° | 23° (1971) | 107° (2001) | 1.34" | 0" |
| November | 40° | 63° | 20° (1916) | 91° (1936) | 3.5" | 0" |
| December | 35° | 54° | 11° (1932) | 76° (2002) | 3.63" | 0.1" |

Site Drainage and Infiltration

To design stormwater management systems, the science of Hydrology and the calculation of runoff for each particular site are necessary considerations.

The hydrology of a storm event is the basis for all aspects of stormwater management design, including pipe size and selecting appropriate treatment devices and methods.

Methods used to determine volumes and peak flows are historical data.

Precipitation and frequency information can be found on the National Oceanic and Atmospheric Administration's National Weather Service website (nws.noaa.gov).

The Rational Method

Many jurisdictions use the Rational Method for calculating peak flow rates.

It translates peak intensity of rainfall directly into peak intensity of runoff.

$$Q = Aci$$

Where

Q = Runoff, cfs

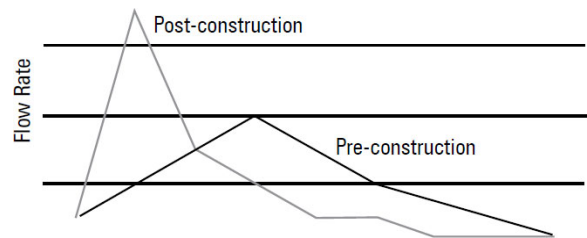
A = Drainage area, acres (1 acre = 43,560 square feet)

C = runoff coefficient, dimensionless (see table below)

i = Rainfall intensity, inches per hour

(To convert acre-inches per hour to cfs, use a correction factor of 1.008.)

| Table Coefficients for Use with the Rational Method | |
|---|------------------------------|
| Type of Surface or Land Use | Runoff Coefficient (c value) |
| Individual soil covers | |
| Forest | 0.1 – 0.3 |
| Turf or meadow | 0.1 – 0.4 |
| Cultivated field | 0.3 – 0.4 |
| Steep grassed area (2:1) | 0.5 – 0.7 |
| Bare earth | 0.2 – 0.9 |
| Gravel or macadam pavement | 0.35 – 0.7 |
| Concrete or asphalt pavement | 0.8 – 0.9 |
| Composite land uses | |
| Flat residential, about 30% impervious | 0.40 |
| Flat residential, about 60% impervious | 0.55 |
| Sloping residential, about 50% impervious | 0.65 |
| Flat commercial, about 90% impervious | 0.80 |

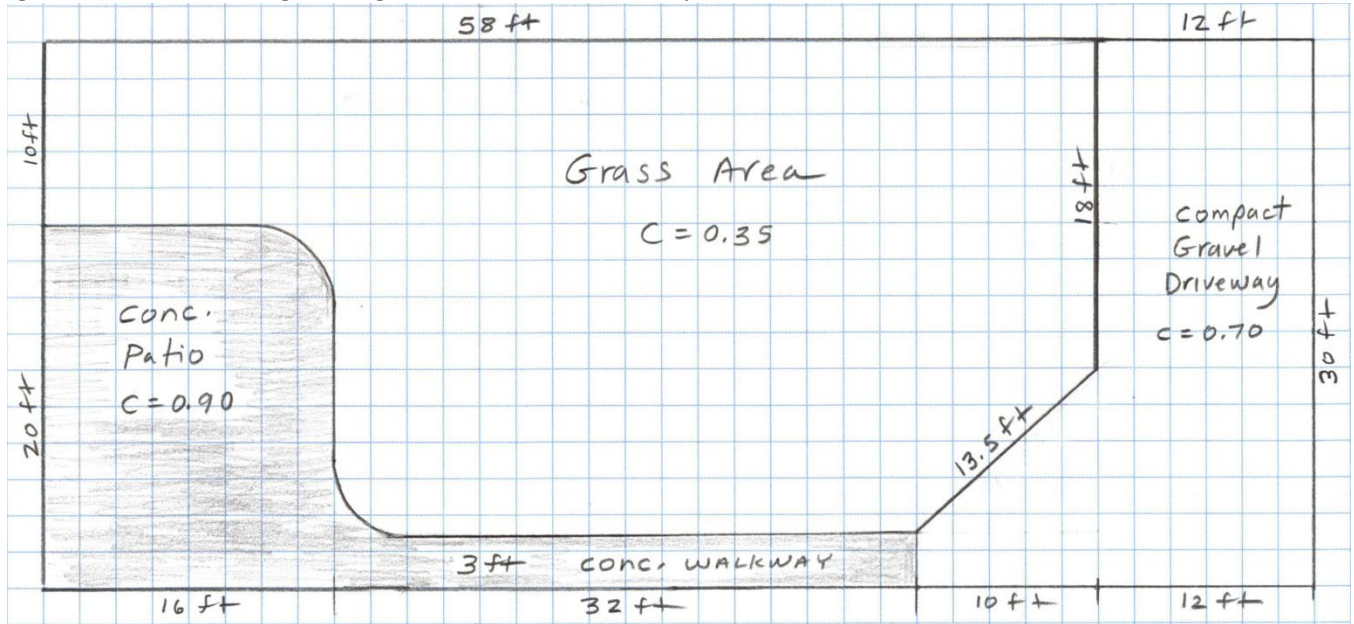


Example 4

Calculate the peak runoff (gpm) for a 1.5-acre site with a concrete or pavement cover and a 4-inches -per-hour rainfall.

Example 5

For the residential site shown use the Rational Method to determine the peak runoff rate (gpm) and volume (gallons) for the drainage area given. The rainfall intensity is 4.2 in/hr.



The Rational Method equation is: $Q = C \times I \times A$ where:

Q = Storm Water Runoff (in cubic feet per second)

C = Coefficient of Runoff

I = Rainfall Intensity (in inches per hour)

A = Area of Drainage Zone (in acres)

The equation above can be modified to give you runoff in gallons per minute.

The modified equation is: $Q = (C \times I \times A) / 96.23$ where:

Q = Storm Water Runoff (in gallons per minute, gpm)

C = Coefficient of Runoff

I = Rainfall Intensity (in inches per hour)

A = Area of Drainage Zone (in square feet)

Concrete Area

Compact Gravel Area

Grass Area