GENERAL NOTES:

(THESE SPECIFICATIONS ARE IN ADDITION TO AND DO NOT EXCLUDE ANY FOUND IN THE GENERAL SPECIFICATIONS FOR THE PROJECT)

1. CONTRACTOR SHALL BE RESPONSIBLE FOR BRACING AND SHORING OF STRUCTURE DURING CONSTRUCTION. FOUNDATION WALLS WHICH ARE TIED TO SLABS OR FLOOR/ROOF FRAMING SHALL BE BRACED AGAINST BACKFILL MOVEMENT UNTIL SLAB/FRAMING (WTIH DECK ATTACHMENT) IS COMPLETED. CONTRACTOR SHALL PROVIDE ALL TEMPORARY SAFETY ENCLOSURES TO PROTECT ALL PERSONNEL INVOLVED IN PROJECT.

2. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR INITIATING, MAINTAINING AND SUPERVISING ALL SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK. KEYSTONE STRUCTURAL ENGINEERING, P.C., IS NOT RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION OR FOR RELATED SAFETY PRECAUTIONS AND PROGRAMS.

3. SHOP DRAWINGS AND SUBMITTALS:
A. CONTRACTOR SHALL FURNISH COMPLETE AND DETAILED SHOP DRAWINGS. ALL SHOP DRAWINGS SHALL BE PREPARED UNDER THE DIRECT SUPERVISION OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED.

B. CONTRACTOR SHALL REVIEW AND VERIFY ALL DIMENSIONS AND STAMP ALL SHOP DRAWINGS PRIOR TO SUBMITTING DRAWINGS TO ARCH/ENG. FOR REVIEW. CONTRACTOR IS RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS SUCH AS, MEMBER SIZES, DETAILS AND DIMENSIONS SPECIFIED IN THE CONSTRUCTION DOCUMENTS.

C. SHOP DRAWINGS SHALL SHOW MEMBERS SIZES, LOCATION, TYPE AND EXTENT OF ALL MEMBERS, BOLTS AND WELDS.

D. CONTRACTOR SHALL HAVE AN APPROVED SHOP DRAWINGS AND PROOF OF WELDER CERTIFICATION AT JOB SITE AT ALL TIMES.

E. PROVIDE SETTING DRAWINGS, TEMPLATES AND DIRECTIONS FOR INSTALLATION OF ANCHOR BOLTS AND OTHER ANCHORAGES TO BE INSTALLED BY OTHERS.

F. COORDINATE ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.

4. DESIGN LOADS:
THE BUILDING STRUCTURE DESCRIBED IN THESE PLANS SHALL BE CONSTRUCTED IN COMPLIANCE WITH THE 2012 INTERNATIONAL BUILDING CODE

A. GRAVITY LOADS
DEAD LOADS:
ROOF: 20 PSF
FLOOR: 60 PSF

LIVE LOADS:
ROOF: 20 PSF
FLOOR: 125 PSF
STAIRS/CORRIDOR: 100 PSF

B: SNOW LOADS:
GROUND SNOW LOAD (Pg): 5 PSF
FLAT ROOF SNOW LOAD (Pf): 5.0 PSF
SNOW EXPOSURE FACTOR (Ce): 1.0
SNOW IMPORTANCE FACTOR (Is): 1.0
THERMAL FACTOR (Ct): 1.0

C. WIND LOADS
BASIC WIND SPEED (3 SEC. GUST): Vult 115 MPH
RISK CATEGORY: II
EXPOSURE CATEGORY: B
INTERNAL PRESSURE (GCpi): +-.18

D. SEISMIC DESIGN CRITERIA:
SEISMIC IMPORTANCE FACTOR (Ie): 1.0
RISK CATEGORY: II
MAPPED SPECTRAL RESPONSE ACCELERATIONS: Ss: 0.185g S1: 0.090g
SITE CLASS: D
S(DS): 0.197
S(D1): 0.091
SEISMIC DESIGN CATEGORY: C
BASIC SEISMIC FORCE RESISTING SYSTEM:
LIGHT FRAMED WALLS SHEATHED WITH STEEL SHEETS
DESIGN BASE SHEAR = 190 KIPS (BOTH DIRECTIONS)
SEISMIC RESPONSE COEFFICIENT (Cs): .0312
RESPONSE MODIFICATION FACTORS (R): 6.5
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE

FOUNDATION NOTES:

1. SEE ARCHITECTURAL AND CIVIL DRAWINGS FOR BUILDING LOCATION AND ORIENTATION. COORDINATE ALL DIMENSIONS WITH ARCH. DRAWINGS. DO NOT SCALE DRAWING

3. SEE ARCHITECTURAL DRAWINGS FOR SIDE WALK EXTENTS, PLANTER, AND PAVER LOCATIONS AND DETAILS.

4. COORDINATE FINISHED FLOOR ELEVATIONS (F.F.E.) WITH ARCH. AND CIVIL DRAWINGS.

5. REFERENCE FFE = SEE PLANS

6. ALL FOOTINGS HAVE BEEN DESIGNED BASED UPON THE RECOMMENDATIONS OUTLINED IN THE REPORT OF PRELIMINARY SUBSURFACE EXPLORATION AND GEOTECHNICAL ENGINEERING EVALUATION BY PIEDMONT GEOTECHNICAL CONSULTANTS, INC. PGC, PROJECT NO. 117096, DATED 04/10/2017.

7. ALL FOUNDATION EXCAVATIONS SHALL BE EVALUATED BY THE GEOTECHNICAL ENGINEER OR TESTING AGENCY PRIOR TO POURING ANY FOUNDATION CONCRETE. CONTACT STRUCTURAL ENGINEER IF SOIL CONDITION ENCOUNTERED DO NOT SATIFY THE RECOMMENDATIONS OUTLINED IN THE REPORT OF PRELIMINARY SUBSURFACE EXPLORATION AND GEOTECHNICAL ENGINEERING EVALUATION.

8. ALL EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 2'-0" BELOW F.F.E. AND A MINIMUM OF 1'-6" BELOW ADJACENT EXTERIOR FINISH GRADE (TYP. UNO)

9. TOP OF INTERIOR FOOTINGS SHALL BE A MINIMUM OF 0-8" BELOW F.F.E. (UNO)

10. PRIOR TO COMMENCING ANY FOUNDATION WORK, COORDINATE WORK WITH ANY EXISTING OR NEW UTILITIES. LOWER FOUNDATION AS REQUIRED TO AVOID INTERFERENCE WITH UTILITIES.

11. INDICATES FOOTING STEP. SEE PLANS

SPECIALTY ENGINEERED PRODUCTS:

1. THE GENERAL CONTRACTOR IS RESPONSIBLE TO COORDINATE THE PROPER SUBMISSION OF SPECIALTY ENGINEERED SHOP DRAWINGS WHICH SHALL BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED. IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO ASSURE THAT THE SPECIALTY ENGINEERED SHOP DRAWINGS ARE SUBMITTED IN A TIMELY MANNER SO AS TO
ALLOW REVIEWS AND RESUBMISSIONS AS REQUIRED. ALL SPECIALTY ENGINEERED PRODUCTS SHALL BE DESIGNED FOR THE APPROPRIATE GRAVITY LOADS AND WIND LOADS INCLUDING UPLIFT AND LATERAL LOADS. INTERIOR SPECIALTY PRODUCTS SHALL BE DESIGNED FOR LATERAL LOADS TO ASSURE STABILITY. SPECIALTY ENGINEERED PRODUCTS SHALL BE, BUT ARE NOT LIMITED TO, THE FOLLOWING:

A. LIGHT GAUGE METAL FRAMING INCLUDING, BUT NOT LIMITED TO, SOFFITS, CLADDING, CEILINGS, ETC.

B. MISCELLANEOUS METALS INCLUDING STEEL STAIRS, MECHANICAL EQUIPMENT SUPPORTS, FRAMES THAT SUPPORT MACHINES, PIPES OR OTHER STRUCTURAL METAL USED FOR SUPPORT OF MECHANICAL SYSTEMS.

C. MISCELLANEOUS HANGARS, METAL FRAMES, LADDERS, RIGGING, HANGING WALLS, METAL RAILINGS, GLAZING FRAMES, CLADDING SUCH AS STONE, PRECAST, ALUMINUM, METAL PANELS, CABLE BARRIER SYSTEMS, ETC. OR ANY OTHER MISCELLANEOUS PRODUCT REQUIRED BY THE ARCHITECTURAL OR MECHANICAL CONSTRUCTION DOCUMENTS.

CONCRETE SLAB ON GRADE NOTES:

1. BASEMENT FLOOR SLAB-ON-GRADE SHALL BE 5" THICK 3000 PSI MINIMUM COMPRESSIVE STRENGTH NORMAL WEIGHT CONCRETE. REINFORCED w/ 6X6-W2.1/W2.1 WWF, LOCATED MID-DEPTH OF SLAB. SEE CIVIL PLANS FOR FINISHED FLOOR ELEVATIONS. COORDINATE VAPOR BARRIER REQUIREMENTS w/ ARCHITECTURAL AND GEOTECHNICAL ENGINEER REQUIREMENTS, PROVIDE 15 MIL (MIN) POLYETHYLENE VAPOR BARRIER WITH JOINTS LAPPED 6" AND TAPED UNDER ALL INTERIOR SLABS. REFER TO GEOTECHNICAL ENGINEER FOR BELOW SLAB ON GRADE, SUBGRADE PREPARATION REQUIREMENTS.

2. CONTROL/CONSTRUCTION JOINTS SHALL BE LOCATED SUCH THAT NO AREA EXCEEDS 400 SQUARE FEET NOR SHALL THE LENGTH EXCEED 1.5 TIMES THE WIDTH. CONSTRUCTION JOINTS MY BE SUBSTITUTED FOR CONTROL JOINTS. METAL " KEYHOLD" JOINTS SHALL BE USED AT ALL CONSTRUCTION JOINTS. LOCATE ALL CONTROL JOINT EQUAL DISTANCE BETWEEN LOAD BEARING WALL AND 5'-0" MIN FROM THE END OF LOAD BEARING WALLS (INCLUDING OPENINGS ETC.)

3. CONDUITS AND PIPES EMBEDDED IN SLABS:
SHALL NOT BE LARGER IN OUTSIDE DIMENSION THAN ONE-THIRD THE OVERALL THICKNESS OF THE SLAB. SHALL NOT BE SPACED CLOSER THAN THREE DIAMETERS OR WIDTHS ON CENTER. A MINIMUM SLAB THICKNESS OF 41/2" MUST BE MAINTAINED OVER THE EMBEDDED CONDUITS OR PIPES.

4. THICKEN SLAB AT LOCATIONS INDICATED ON FOUNDATION PLAN SEE 6/S3.1.

STEEL NOTES:

1. STRUCTURAL STEEL:
A. SHALL CONFORM TO THE LATEST STANDARDS OF ASTM:
WIDE FLANGE BEAMS: A992
MISC. STRUCTURAL STEEL SHAPES, PLATES AND BARS: A36
HOLLOW STRUCTURAL STEEL SECTIONS (ROUND AND RECTANGULAR):
A500 GRADE B
STRUCTURAL STEEL PIPE: A53 GRADE B

B. STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE STANDARDS AND SPECIFICATIONS OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC 360-05) USING ALLOWABLE STRESS DESIGN.

C. PROVIDE 1" (MINIMUM) NON-SHRINK GROUT UNDER ALL BASE PLATES.

D. SHOP OR FIELD SPLICES BETWEEN SUPPORTS THAT ARE NOT REQUIRED BY DESIGN WILL NOT BE ALLOWED. ANY MEMBERS CONTAINING SUCH SPLICES FOUND IN THE FIELD SHALL BE REMOVED AND REPLACED WITH UNSPLICED MEMBERS AT THE FABRICATOR'S EXPENSE.

2. STEEL CONNECTIONS:
A. WHERE BEAM REACTIONS OR DETAILS ARE NOT SHOWN IN THE CONSTRUCTION DOCUMENTS. CONNECTIONS SHALL BE DESIGNED FOR ONE-HALF THE MAXIMUM (SIMPLE SPAN) UNIFORM LOAD WHICH THE MEMBER WILL SUPPORT FOR THE SPAN SHOWN ON THE DRAWINGS.

B. BOLTS SHALL BE HIGH STRENGTH A-325 BOLTS OF SAME SIZE AND NUMBER AS SHOWN ON DRAWINGS. CONNECTIONS SHALL CONFORM TO THE SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A-325 OR A-490 BOLTS. CONNECTIONS ARE BEARING TYPE.

C. BOLTS SHALL BE TIGHTENED TO THE SNUG TIGHT CONDITION UNLESS OTHERWISE NOTED ON THE DRAWINGS.

3. WELDS:
A. MINIMUM SIZE OF WELD IS 3/16" AND (E70XX) UNLESS NOTED OTHERWISE.

B. ALL WELDING SHALL CONFORM TO THE LATEST "STRUCTURAL WELDING CODE" BY THE AMERICAN WELDING SOCIETY. ALL WORK SHALL BE PERFORMED BY CERTIFIED WELDERS EXPERIENCED IN THE TYPE OF CONSTRUCTION INVOLVED. PROOF OF WELDER CERTIFICATION SHALL BE AVAILABLE AT THE JOB SITE.

4. PROVIDE ALL SUPPORTING STEEL NOT INDICATED ON PLAN AS REQUIRED FOR THE INSTALLATION OF MECHANICAL EQUIPMENT AND MATERIALS, INCLUDING ANGLES, CHANNELS, BEAMS, HANGERS, ETC. DO NOT SUPPORT EQUIPMENT OR PIPING FROM METAL DECKING.

5. STEEL STAIRS:
ENGINEERED CONCRETE FILLED STEEL STAIR SYSTEM AND ALL CONNECTIONS OF THE SAME TO THIS STRUCTURE SHALL BE DESIGNED BY AN ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED. SUBMIT SHOP DRAWINGS BEARING THE SEAL AND THE SIGNATURE OF THE ENGINEER FOR REVIEW PRIOR TO FABRICATION. THE CONFIGURATION OF THE STEEL STAIR SYSTEM SHALL BE SHOWN ON THE ARCHITECTURAL DRAWINGS. STEEL STAIR SYSTEM AND ALL CONNECTIONS SHALL BE DESIGNED FOR ALL APPLICABLE LOADS AS INDICATED ON THE PLANS AND IN ALL APPLICABLE BUILDING CODES. THE LOADS SHALL BE CLEARLY INDICATED ON ALL SHOP DRAWINGS. SHOP DRAWINGS SHALL SHOW AND SPECIFY ALL CONNECTIONS UTILIZED WITHIN THE STEEL STAIR SYSTEM AS WELL AS CONNECTIONS TO AND LOADS IMPOSED UPON THE STRUCTURAL SYSTEM SHOWN OF THESE PLANS.

COLD FORMED METAL FRAMING NOTES:

1. METAL FRAMING;
NON-STANDARD SSMA METAL FRAMING MEMBER (CEE (C), TRACK, ZEE (Z) & HAT CHANNELS) DIMENSIONS, CONFIGURATIONS AND SECTION PROPERTIES SHALL COMPLY WITH THE 2007 AISI, COLD-FORMED STEEL DESIGN SPECIFICATIONS

2. USE STEEL TO FORM ZINC COATED (GALVANIZED) NON-STANDARD SSMA METAL FRAMING MEMBERS TO MEET THE PHYSICAL AND CHEMICAL PROPERTIES OF ASTM A653, GRADE 55 (FY=55 KSI), AND G60 COATING DESIGNATION AS DESCRIBED IN ASTM A 924

3. USE STEEL TO FORM PRIMED NON-STANDARD SSMA METAL FRAMING MEMBERS TO MEET THE PHYSICAL AND CHEMICAL PROPERTIES OF ASTM A1011, GRADE 55 (FY=55 KSI).

4. SUBMIT SHOP DRAWING INDICATING MEMBER DIMENSIONS, CONFIGURATIONS, SECTION PROPERTIES, COATING, DETAIL OF FABRICATION, DETAILS OF ATTACHMENT TO ADJOINING WORK AND DETAILS OF ALL FASTENERS INCLUDING LOCATION AND SPACING OF ALL FASTENERS ATTACHING FRAMING TO ITSELF.

5. SHOP DRAWING SUBMITTAL TO INCLUDE PLANS, ELEVATIONS, SECTIONS AND DETAILS

6. SUBMIT SHOP DRAWINGS INCLUDING CALCULATIONS PREPARED, SIGNED AND SEALED BY A ENGINEER LICENSED WITH AND ACTIVE REGISTRATION IN THE STATE WHERE THE PROJECT IS LOCATED.

7. SCREWS SHALL BE SELF DRILLING TYPE SCREWS OF THE OF THE MINIMUM DIAMETER INDICATED ON PLANS AND DETAILS WITH A MIN PENETRATION THROUGH JOINED MATERIAL OF NOT LESS THAN (3) EXPOSED THREADS.

8. SCREW SPACING AND EDGE DISTANCES SHALL BE (3) SCREW DIAMETERS WITH A MIN 1/2" DIMENSION.

9. SCREW SHALL HAVE A PROTECTIVE COATING OF NOT LESS THAN THE JOINED MEMBER PROTECTIVE COATING.

10. ALL BOLTS FOR CONNECTIONS SHALL CONFORM TO ASTM A325

11. DO NOT ALTER, CUT OR REMOVE MEMBERS OR CONNECTIONS OF MEMBERS WITH PRIOR WRITTEN APPROVAL OF ENGINEER.

12. FIELD TOUCH UP ALL FIELD WELDS AND ABRASIONS OF GALVANIZED MATERIALS WITH ZINC RICH PAINT IN ACCORDANCE WITH ASTM A 780, ANNEX A2. TOUCH UP WORK SHALL BE COMPLETED PRIOR TO ATTACHMENT OF THE WORK OF ANY OTHER SECTIONS TO THE LIGHT GAUGE STEEL FRAMING.

13. STANDARD SSMA METAL STUD AND TRACK FRAMING

A. METAL STUD STRENGTH CRITERIA:
1. 18 MIL TO 43 MIL - 33 KSI MIN. YIELD STRESS
2. 54 MIL TO 97 MIL - 50 KSI MIN. YIELD STRESS
3. RUNNER TRACK - 33 KSI MIN. YIELD STRESS (UNO)
DO NOT USE STUDS LESS THAN 43 MIL FOR ANY EXTERIOR WALL USED TO BACK-UP BRICK VENEER.

B. ATTACH METAL FRAMING TO PRIMARY STRUCTURE WITH A DEFLECTION TRACK OR A STEEL NETWORK INC. VERTICAL DEFLECTION CLIPS CAPABLE OF ALLOWING 1" OF VERTICAL DEFLECTION.

14. METAL STUD FASTENERS:
A. SCREW CONNECTIONS
USE #10-16 KWIK-FLEX SELF-DRILLING SCREWS OR APPROVED EQUAL UNLESS OTHERWISE NOTED. 4 SCREWS PER C CONNECTION MIN., UNLESS NOTED OTHERWISE OR A PRE-ENGINEERED TRUSS.
B. POWER DRIVEN FASTENERS:

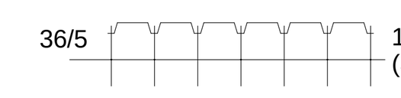
1. FASTENING TO CONCRETE:
USE 0.145" DIA. DOME HEAD NAIL TYPE "X-ZF" BY HILTI OR APPROVED EQUAL UNLESS NOTED OTHERWISE.
MIN. EMBEDMENT = 1 1/4" MIN. EDGE DISTANCE = 2"
MIN. SPACING = 3" MAX. SPACING = 12"

2. FASTENING TO STEEL:
USE 0.145" DIA. DOME HEAD KNURLED SHANK FASTENER TYPE "X-EDNI" BY HILTI OR APPROVED EQUAL UNLESS NOTED OTHERWISE.
MIN. EMBEDMENT = FULL PENETRATION
MIN. EDGE DISTANCE = 1/2"
MIN. SPACING = 1 1/2" MAX. SPACING 12"

ROOF FRAMING NOTES:

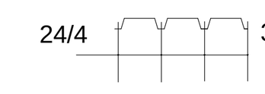
1. DESIGN ROOF JOIST FOR A NET UPLIFT OF 20 PSF (ASD) (UNO). NO 1/3 STRESS INCREASE ALLOWED.

2. 1.5" TYPE B ROOF DECK: (ALL CANOPIES U.N.O.)



ATTACH CANOPY DECK TO SUPPORTS WITH (#12 SCREWS) IN A 36/4 PATTERN WITH 6 SIDELAPS FASTENERS (#10 SCREWS) PER SPAN. ATTACHMENT AT PERIMETER OF DECK SHALL BE EQUAL TO ATTACHMENT AT DECK SHEET LAPS AND DECK SHEET ENDS. ANY PARTIAL OR SKEWED SHEETS SHALL BE ATTACHED AT EVERY FLUTE REGARDLESS OF ATTACHMENT PATTERN.

3. 3" TYPE N ROOF DECK: (MAIN ROOF)



ATTACH ROOF DECK TO SUPPORTS WITH (#12 SCREWS) IN A 24/4 PATTERN WITH 7 SIDELAP FASTENERS (#10 SCREWS) PER SPAN. ATTACHMENT AT PERIMETER OF DECK SHALL BE EQUAL TO ATTACHMENT AT DECK SHEET LAPS AND DECK SHEET ENDS. ANY PARTIAL OR SKEWED SHEETS SHALL BE ATTACHED AT EVERY FLUTE REGARDLESS OF ATTACHMENT PATTERN.

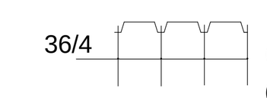
4. ROOF SYSTEM IS DESIGNED FOR MECHANICAL EQUIPMENT SHOWN. NOTIFY ARCH/ENGINEER IF ADDITIONAL EQUIPMENT REQUIRED OR IF HEAVIER UNITS ARE SUPPLIED.

5. COORDINATE THE EXACT LOCATION AND EXTENT OF ROOF HATCH OPENINGS WITH ARCH. DRAWINGS.

FLOOR FRAMING NOTES:

3. FLOOR SLAB SHALL BE 51/2" (TOTAL THICKNESS) WITH NORMAL WEIGHT CONCRETE (f`c=3000 PSI) ON 3" 20 GAUGE COMPOSITE METAL DECK (50 Fy) (NEW MILLENNIUM 3.0 CD OR EQUAL). REINFORCE SLAB WITH WWR: 6X6-W1.4XW1.4 LOCATED AT MID-DEPTH OF SLAB.

4. ATTACHMENT OF FLOOR DECK TO SUPPORTS WITH (#12 TEK SCREWS) IN A 36/4 PATTERN WITH 4 SIDE LAPS PER SPAN (#10 SCREWS). ATTACHMENT AT PERIMETER OF DECK SHALL BE EQUAL TO ATTACHMENT AT DECK SIDE LAPS AND DECK SHEET ENDS. ANY PARTIAL OR SKEWED SHEETS SHALL BE ATTACHED AT EVERY FLUTE REGARDLESS OF ATTACHMENT PATTERN.



5. SUSPENDED CEILINGS, LIGHT FIXTURES, DUCTS AND OTHER PERMANENT SUSPENDED LOADS SHALL NOT BE SUPPORTED BY THE METAL DECKING.

6. PROVIDE 51/2"x5" LLV 14GA CONT. POURSTOP ANGLE AT ALL DECK EDGES AND AROUND ALL OPENINGS IN FLOOR SLAB.

7. PROVIDE 6"x4"x1/4" LLV ANGLE AT ALL DECK EDGES SUPPORTED ON CMU WALLS.

SPECIAL INSPECTIONS:

A. THE SPECIAL INSPECTOR SHALL BE ENGAGED BY THE OWNER. SPECIAL INSPECTOR SHALL BE FULLY QUALIFIED, APPROVED BY THE BUILDING OFFICIAL, REGISTERED BY APPLICABLE REGISTRATION BOARD IF REQUIRED BY THE LOCAL BUILDING OFFICIAL, AND SHALL BE ACCEPTABLE TO THE ARCHITECT.

B. THE SPECIAL INSPECTOR SHALL PROVIDE VERIFICATION OF CONSTRUCTION QUALITY CONTROL INSPECTIONS AND TESTING. THE SPECIAL INSPECTOR SHALL CERTIFY THAT ALL WORK REQUIRING INSPECTION IS PERFORMED IN COMPLIANCE WITH ALL REQUIREMENTS OF THE CONTRACT DOCUMENTS, BUILDING CODE REQUIREMENTS AND LOCAL BUILDING DEPARTMENT REQUIREMENTS.

C. SPECIAL INSPECTIONS ARE REQUIRED FOR THE ITEMS NOTED IN THE STATEMENT OF SPECIAL INSPECTIONS AND THE 2012 IBC CHAPTER 17. THE CONTRACTOR SHALL OBTAIN A COPY OF THE STATEMENT OF SPECIAL INSPECTIONS AND NOTIFY THE SPECIAL INSPECTOR WHEN WORK IS READY TO BE INSPECTED.

D. FAILURE TO NOTIFY THE SPECIAL INSPECTOR PRIOR TO OBSCURING AN ITEM REQUIRING INSPECTION MAY RESULT IN THE CONTRACTOR REMOVING OTHER WORK TO ALLOW INSPECTION. THIS WORK WILL BE REMOVED AND REPLACED AT THE CONTRACTORS EXPENSE. FAILURE TO HAVE REQUIRED ITEMS INSPECTED IS REASON FOR REJECTION OF THE WORK.

E. PREMATURE NOTIFICATION FOR INSPECTIONS WILL RESULT IN ADDITIONAL INSPECTION WITH ALL EXPENSES AND FEES PAID FOR BY THE CONTRACTOR.

SHEAR WALL AND WALL PANEL NOTES

1. 26 GAUGE PBU WALL PANELS WHERE NOTED, ALL OTHER PANELS SHALL BE 29 GAUGE PBU WALL PANELS. ALL WALL PANELS SHALL BE GRADE 80

2. ATTACH ALL PBU PANEL ENDS TO SUPPORTS WITH #12 TEK SCREWS AT 6" o.c. AND TO INTERMEDIATE SUPPORTS AT 12" o.c.

3. SIDELAP ATTACHMENT FOR WALLS DESIGNATED AS SHEAR WALLS ON THE PLANS SHALL BE #12 TEK SCREWS AT 12" o.c. (20" o.c. FOR ALL OTHER WALL). FOR WALLS DESIGNATED AS SHEAR WALLS ON THE PLANS, FASTEN BOTTOM EDGE OF PANEL TO BOTTOM TRACK WITH #12x3/4" TEK SCREW PANEL FASTENERS @ 12" o.c. (20" o.c. FOR ALL OTHER WALL).

4. ALL PERIMETER PANELS SHALL BE FULL HEIGHT 29 GAUGE PBU WALL PANELS

5. ALL INTERIOR PANELS SHALL BE 8'-4" HIGH. COORDINATE EXACT PANEL HEIGHT WITH ARCHITECTURAL.

6. THERE SHALL BE A DOUBLE STUD AT EACH END OF EACH SHEAR WALL AT THE LEVEL OF THE SHEAR WALL AND ALL LEVELS BELOW WITH THE EXCEPTION OF SHEAR WALLLS AT THE ROOF LEVEL. THE DOUBLE STUD SIZE, TYPE AND GAUGE SHALL BE EQUAL TO THE SHEAR WALL FRAMING MEMBERS AT EACH LEVEL.

7. SCREWS SHALL BE SELF DRILLING TYPE SCREWS WITH A MIN PENETRATION THROUGH JOINED MATERIAL OF NOT LESS THAN (3) EXPOSED THREADS.

8. SHEAR WALLS DESIGNATED ON A LEVEL ARE THE WALLS BELOW THAT LEVEL.

SW-1 29 GAUGE PBU PANEL ON ONE SIDE OF WALL TO 8'-4" MINIMUM WITH STRAP BRACES ABOVE TOP OF PANEL

SW-2 26 GAUGE PBU PANEL ON ONE SIDE OF WALL TO 8'-4" MINIMUM WITH STRAP BRACES ABOVE TOP OF PANEL

REINFORCED MASONRY NOTES:

IMPORTANT;
SPECIAL INSPECTION IS REQUIRED FOR ALL MASONRY CONSTRUCTION.

1. ALL MASONRY WALLS SHALL HAVE CELLS REINFORCED AND DOWELED INTO FOUNDATION AS NOTED ON THE DRAWINGS. ALL MASONRY BELOW GRADE SHALL BE GROUTED SOLID.

2. REINFORCED CELLS WITH GROUT SHALL BE CONTINUOUS FROM FOUNDATION TO THE TOP OF THE WALL.

3. FILL REINFORCED CELLS WITH MECHANCIALLY MIXED GROUT (2,500 PSI MIN.).

A. GROUT SHALL CONFORM TO ASTM C-476 (PROPORTION SPECIFICATION).
B. GROUT SHALL BE MIXED FOR AT LEAST 5 MINUTES IN MECHANICAL MIXER AND PLACED WITHIN 1 1/2 HOURS FROM THE INITIAL INTRODUCTION OF WATER AND PRIOR TO INTIAL SET.
C. BETWEEN GROUT POURS, A HORIZONTAL CONSTRUCTION JOINT SHALL BE FORMED BY STOPPING ALL CMU AT THE SAME ELEVATION AND WITH THE GROUT STOPPING A MINIMUM OF 1 1/2" BELOW A MORTAR JOINT, EXCEPT AT THE TOP OF THE WALL. WHERE BOND BEAMS OCCUR, THE GROUT POUR SHALL BE STOPPED A MINIMUM OF 1/2" BELOW THE TOP OF THE MASONRY.
D. CMU WALLS SHALL BE CONSTRUCTED USING LOW-LIFT GROUTING (5'-0" MAX. POUR HEIGHTS).
E. ALL EXTERIOR WALLS AND INTERIOR SHEAR WALLS ARE MARKED ON THE PLANS AS (SW). SEE SCHEDULE ON THIS SHEET FOR REINFORCEMENT.

4. ALL CONCRETE MASONRY SHALL HAVE A MINIMUM NET AREA COMPRESSIVE STRENGTH (f`m) OF 1500 PSI. (MSJC S1.4B.2B TABLE 2).

5. MORTAR SHALL BE TYPE S-(ASTM 270 TABLE 1 MORTAR PROPORTION SPECIFICATION REQUIREMENTS).
A. THICKNESS OF BED JOINTS SHALL BE 3/8" EXCEPT THAT THE THICKNESS OF THE BED JOINT OF THE STARTING COURSE PLACED OVER FOUNDATIONS SHALL NOT BE LESS THAN 1/4" AND NOT MORE THAT 3/4".
B. MORTAR SHALL BE MIXED UNTIL MIXTURE IS UNIFORM THOUGHOUT.
C. UNUSED MORTAR SHALL BE DISCARDED WITHIN 2 1/2 HOURS AFTER INITIAL MIXING.

6. ALL MASONRY UNITS SHALL CONFORM TO ASTM C-90 AND HAVE A NET AREA COMPRESSIVE STRENGTH OF CONCRETE MASONRY UNITS EQUAL TO 1900 PSI (MSJC S1.4B.2B TABLE 2)
A. ALL WALLS SHALL BE CONSTRUCTED USING RUNNING BOND (U.N.O).

7. REINFORCING STEEL SHALL CONFORM TO ASTM A615-GRADE 60. FOR WELDED REINFORCING USE ASTM A706-GRADE 60.

8. ALL MASONRY WALLS SHALL HAVE STANDARD HORIZONTAL (9 GA.) LADDER TYPE REINFORCING @ 16" o.c. JOINT REINFORCEMENT SHALL CONFORM TO ASTM A951.

9. COORDINATE REQUIRED TYPE, SIZE, GAUGE, AND SPACING OF STEEL ANCHORS FOR ATTACHMENT OF MASONRY VENEER WITH ARCH. DRAWINGS.

10. CONTROL JOINTS IN CONCRETE MASONRY SHALL HAVE A MAXIMUM SPACING OF 25'-0" o.c. (UNO). COORDINATE THE LOCATION OF BRICK EXPANSION JOINTS WITH ARCH. DWGS.

11. SLEEVE ALL PLUMBING OR FIRE PROTECTION PIPING THRU CMU WALLS.

12. REINFORCING BARS FOR VERTICAL FILLED CELLS SHALL BE LAPPED ACCORDING TO MASONRY LAP LENTH TABLE 1, SEE THIS SHEET.

13. COORDINATE EXACT LOCATION AND EXTENT OF ALL OPENINGS IN MASONRY WALLS WITH ARCH. DRAWINGS.

14. MINIMUM MASONRY COVER FOR REINFORCING STEEL:
A. MASONRY EXPOSED TO WEATHER OR EARTH:
2" FOR BARS LARGER THAN #5
11/2" FOR #5 AND SMALLER BARS

B. MASONRY NOT EXPOSED TO WEATHER OF EARTH: 11/2"

REINFORCED CONCRETE NOTES:

1. STRUCTURAL MEMBERS OF REINFORCED CONCRETE SHALL BE CONSTRUCTED IN ACCORDANCE WITH ACI318.

2. ALL CONCRETE SHALL HAVE A SLUMP OF 4" (+/-1") AND A MINIMUM 28 DAY COMPRESSIVE STRENGTH:
A. FOOTINGS: 3000 PSI (0.50 MAXIMUM WATER/CEMENT RATIO).
B. WALLS: 3000 PSI (0.50 MAXIMUM WATER/CEMENT RATIO).
C. SLAB ON GRADE: 3000 PSI (540 LBS/CUBIC YARD MINIMUM CEMENTITIOUS MATERIAL) (0.50 MAXIMUM WATER/CEMENT RATIO).

3. ALL STEEL REINFORCEMENT SHALL BE ASTM 615-GRADE 60. ALL WELDED STEEL REINFORCEMENT SHALL BE ASTM A706-GRADE 60. WELDED WIRE REINFORCEMNET SHALL BE ASTM A185. ALL WELDED REINFORCEMENT SHALL BE IN ACCORDANCE WITH AWS D1.4.

4. MINIMUM CONCRETE COVER FOR REINFORCING STEEL: (UNO)

A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3"

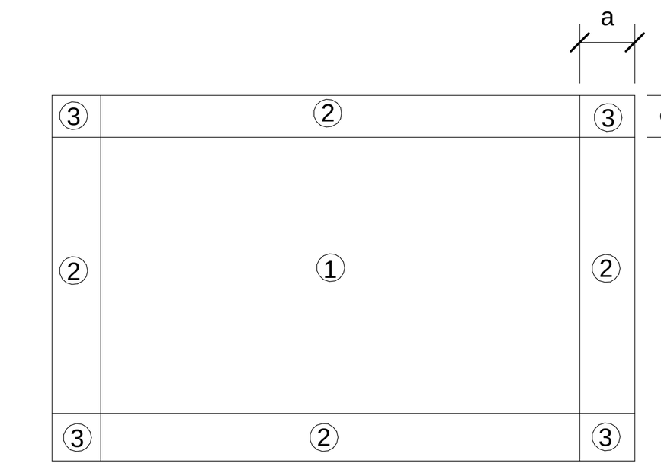
B. CONCRETE EXPOSED TO EARTH AND WEATHER:
#5 REBAR AND SMALLER: 1 1/2"
#6 REBAR AND LARGER: 2"

C. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH EARTH.
SLABS, WALLS, AND JOISTS:
#14 OR #18 REBARS: 1 1/2"
#11 REBAR AND SMALLER: 3/4"
BEAMS AND COLUMNS: 1 1/2"

5. UNLESS NOTED OTHERWISE, CAST IN PLACE CONCRETE SHALL HAVE THE FOLLOWING STEEL ADDED AROUND ALL OPENINGS:
2-#5 (LENGTH OF OPENING+ 48") ALONG EACH SIDE OF OPENING AND TWO (2)-#5x5'-0" DIAGONALLY AT EACH CORNER.

6. ALL CONCRETE COMPRESSIVE STRENGTH TESTS SHALL BE AVAILABLE AT JOB SITE.

7. ALL LAP SPLICE LENGTHS SHALL BE AS SHOWN IN TABLE 2 THIS SHEET



ROOF PRESSURE ZONES

WALL PRESSURE ZONES

COMPONENT AND CLADDING (MAIN ROOF)

ZONE	AREA (SF)	MAX (+) (PSF)	MAX (-) (PSF)	REMARKS
ZONE 1	10	+16.0	-44.08	
	100	+16.0	-40.31	
ZONE 2	10	+17.98	-73.95	
	100	+16.0	-47.85	
ZONE 3	10	+17.98	-111.36	
	100	+16.0	-47.85	

COMPONENT AND CLADDING (WALLS)

ZONE	AREA (SF)	MAX (+) (PSF)	MAX (-) (PSF)	REMARKS
ZONE 4	10	+44.08	-47.85	
	500	+32.92	-36.54	
ZONE 5	10	+44.08	-55.1	
	500	+32.92	-36.54	

WIND TABLE NOTES:
1. LOADS BASED ON ASCE 7-10
2. a=13 FT

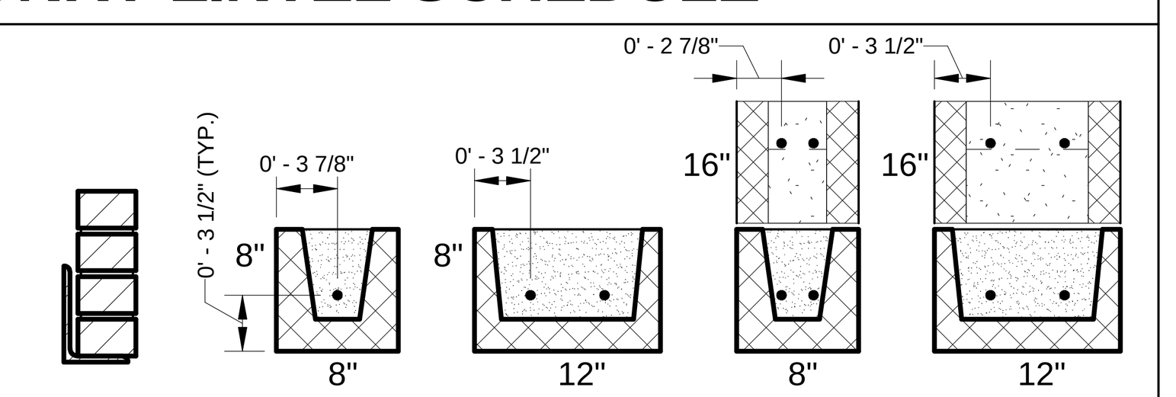
REINFORCED CONCRETE TENSION LAP SPLICE LENGTHS TABLE 2 (INCHES)

BAR SIZE	f`c=3000 PSI	f`c=4000 PSI
#3	25	21.3
#4	33	29
#5	41	36
#6	49	43
#7	72	62

LIST OF ABBREVIATIONS

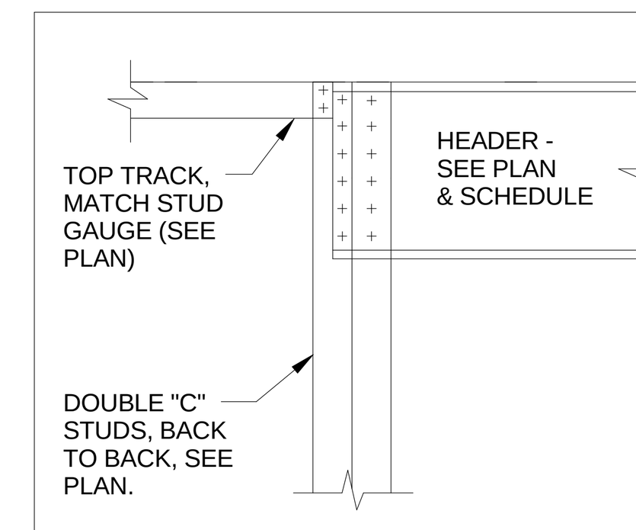
FOB: FACE OF BRICK	LO: LOW CONDITION
FOA: FACE OF ANGLE	HI: HIGH CONDITION
FOS: FACE OF STUD	TOW: TOP OF WALL
TOB: TOP OF BEAM	SDS: SELF DRILLING SCREW
BOC: BOTTOM OF CHANNEL	FOC: FACE OF CHANNEL
T&B: TOP AND BOTTOM	TOF: TOP OF FOOTING
TOA: TOP OF ANGLE	SIM: SIMILAR CONDITION
BOA: BOTTOM OF ANGLE	W.P.: WORKING POINT
LLV: LONG LEG VERTICAL	UNO.: UNLESS NOTED OTHERWISE
LLH: LONG LEG HORIZONTAL	B. ?: BASE PLATE
SW: MASONRY SHEAR WALL	PAF: POWER ACTUATED FASTENERS
TOS: TOP OF SLAB	S.F: STEP FOOTING
T/COL: TOP OF COLUMN	B/COL: BOTTOM OF COLUMN
	FOT: FACE OF TUBE

MASONRY LINTEL SCHEDULE



OPENING WIDTH						
MINIMUM	MAXIMUM	LEDGE ANGLE	8" CMU	12" CMU	8" CMU	12" CMU
-	3'-4"	L3 1/2 x 3 1/2 x 1/4	2 - #4	2 - #4	-	-
3'-4"	5'-4"	L3 1/2 x 3 1/2 x 1/4	2 - #5	2 - #5	-	-
5'-4"	7'-4"	L5 x 3 1/2 x 1/4	-	-	4- #5	4 - #5
7'-4"	10'-0"	L6 x 4 x 3/8	-	-	4- #6	4 - #6
10'-0"	13'-0"	L7 x 4 x 3/8	-	-	-	-

1. EXTEND BOND BEAM REINFORCING 24" or 40 Db BEYOND THE EXTENTS OF THE OPENING. VERTICAL REINFORCING AT THE SIDES OF THE OPENING SHALL BE CONTINUOUS THROUGH THE BOND BEAM. PROVIDE KNOCK OUTS IN THE BOTTOM OF THE BOND BEAM BLOCK AS REQUIRED TO ALLOW REINFORCING TO PASS THROUGH.
2. SEE DETAIL 2/S4.3 FOR ADDITIONAL REINFORCING AT OPENINGS.
3. PROVIDE 8" BEARING FOR STEEL ANGLE LINTELS AT EACH END.



HEADER CONNECTION SCHEDULE

HEADER	HEADER ATTACHMENT EACH SIDE OF DOUBLE STUD
(2) C4 x 2 1/2" x 16 GA	(2) ROWS (3) #12 SCREWS (6 TOTAL)
(2) C4 x 2" x 12 GA	(2) ROWS (3) #12 SCREWS (6 TOTAL)
(2) C6 x 2 1/2" x 14 GA	(2) ROWS (4) #12 SCREWS (8 TOTAL)
(2) C8 x 2 1/2" x 12 GA	(2) ROWS (5) #12 SCREWS (10 TOTAL)
(2) C10 x 2 1/2" x 12 GA	(2) ROWS (7) #12 SCREWS (14 TOTAL)
(2) C12 x 4" x 12 GA	(2) ROWS (7) #12 SCREWS (14 TOTAL)

ISSUED FOR CONSTRUCTION

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GEORGIA REGISTERED No 30737 PROFESSIONAL ENGINEER BRENDAN F. CROWLEY

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CLIENT
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Sandy Springs, GA 30328

SHEET TITLE
GENERAL NOTES & SCHEDULES

Date:
10-01-2018

PROJECT NUMBER
18-115

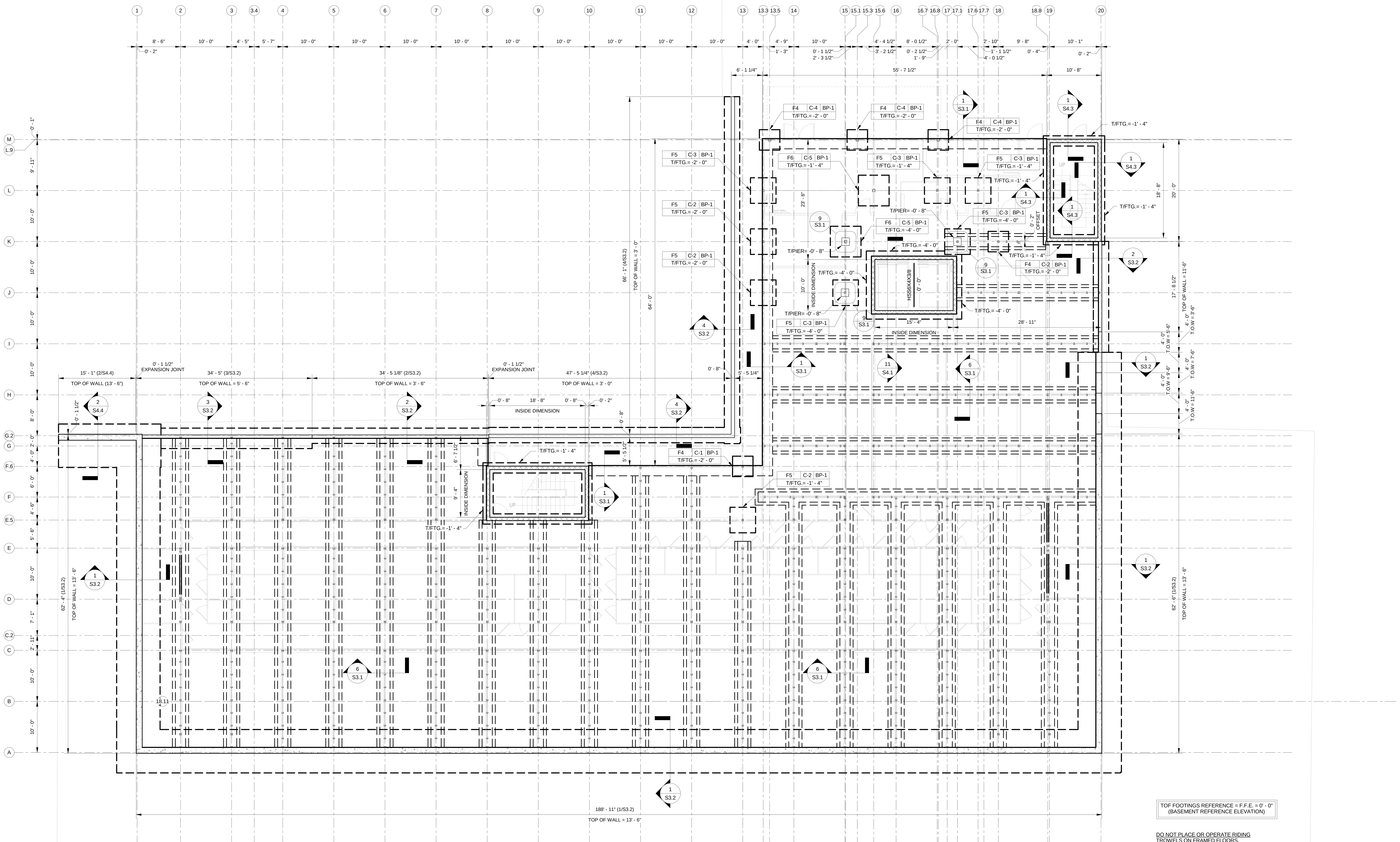
SHEET NUMBER
S0.1

Structural Column Schedule

Type Mark	Type	Type Comments
C-1	HSS4X4X1/4	
C-2	HSS4X4X3/8	
C-3	HSS4X4X1/2	
C-4	HSS6X6X1/4	
C-5	HSS6X6X3/8	
C-6	HSS8X4X3/8	

Structural Foundation Schedule

Type Mark	Footing Dimensions	Reinforcing
F4	4'-0"x4'-0"x1'-0"	(4) #5 EACH WAY
F5	5'-0"x5'-0"x1'-3"	(6) #5 EACH WAY
F6	6'-0"x6'-0"x1'-3"	(7) #5 EACH WAY



TOF FOOTINGS REFERENCE = F.F.E. = 0' - 0"
(BASEMENT REFERENCE ELEVATION)

DO NOT PLACE OR OPERATE RIDING TROWELS ON FRAMED FLOORS.

1 S1.1 Foundation Plan
1/8" = 1'-0"

ISSUED FOR CONSTRUCTION

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GEORGIA REGISTERED PROFESSIONAL ENGINEER BRENDAN F. CROWLEY

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SHEET TITLE
FOUNDATION PLAN

Date:
10-01-2018

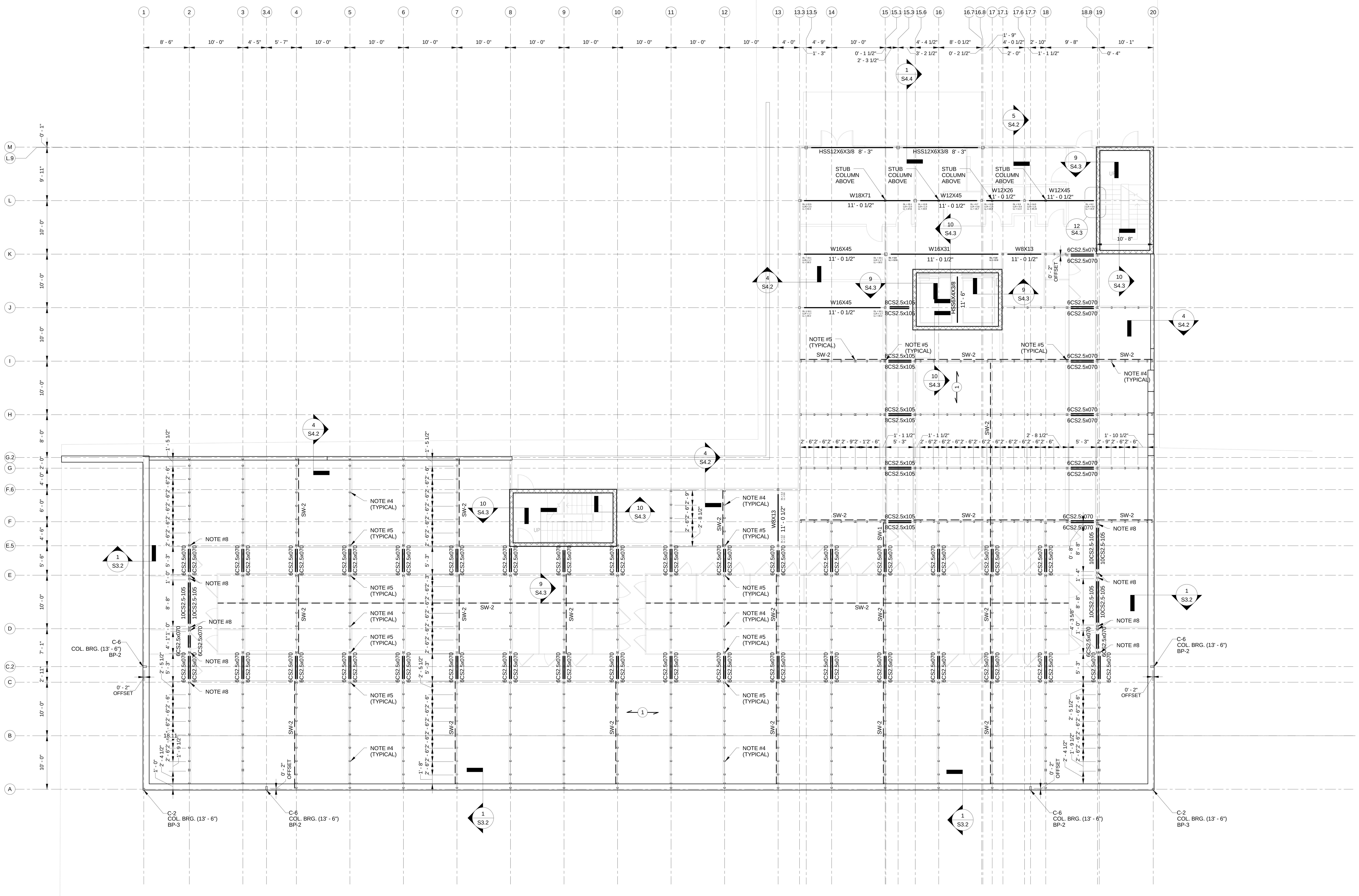
PROJECT NUMBER
18-115

SHEET NUMBER
S1.1

1. CONCRETE FLOOR SLAB TO BE: 51/2" TOTAL 3,000 PSI CONCRETE SLAB W/ 6"x6" W1.4xW1.4 WWF AND 3", 20 GAUGE VLI COMPOSITE DECK (OR EQUAL) W/ #12 TEK SCREWS IN 36/4 PATTERN & (4) #10 TEK SCREWS AT SIDELAPS, TYPICAL, 50 KSI = Fy
2. (2) 6CS2.5x070 HEADER (SEE HEADER CONNECTION SCHEDULE)
3. (2) 10CS2.5x105 HEADER (SEE HEADER CONNECTION SCHEDULE).
4. 6CS2.5x105 "CEE" TYPE METAL STUDS @ 30" O.C. AT ALL INTERIOR BEARING WALL (BELOW), TYPICAL.
5. (2) 6CS2.5x105 DOUBLE STUDS (BACK TO BACK) AT ALL INTERIOR BEAM ENDS, (BELOW), U.N.O.
6. ALL INTERIOR DOORS AND NON-BEARING PARTITION WALLS BY OTHERS, TYP.
7. USE 6CS2.5x070 GAUGE "C" TYPE METAL STUDS AT 24" O.C. AT ALL EXTERIOR WALLS BELOW, (TYP). PROVIDE DOUBLE FULL HEIGHT STUD AT EACH AT ALL WALL OPENINGS.
8. (2) 6CS2.5x105 DOUBLE STUDS (BACK TO BACK) AT EACH ROLL-UP DOOR JAMB, (BELOW), U.N.O.
9. (2) 12CS4.0x105 HEADER (SEE HEADER CONNECTION SCHEDULE).
10. PROVIDE ELEVATOR RAIL ATTACHMENT AT EACH FLOOR LEVEL (CAST INTO FLOOR SLAB) TYPICAL.
11. TRANSVERSE NON-LOAD BEARING SHEAR WALLS SHALL BE 4CS2.0x059 "C" TYPE METAL STUDS @ 2'-6" o.c. TYPICAL
12. (2) 10CS2.0-105 BOX HEADER w/ (12) #12 TEK SCREWS AT EACH END ON EACH SIDE.

ALTERNATE;
CONCRETE FLOOR SLAB TO BE: 4 1/2" TOTAL 3,000 PSI CONCRETE SLAB W/ 6"x6" W1.4xW1.4 WWF AND 2", 18 GAUGE VLI COMPOSITE DECK (OR EQUAL) W/ #12 TEK SCREWS IN 36/4 PATTERN & (4) #10 TEK SCREWS AT SIDELAPS, TYPICAL, 50 KSI Fy

DO NOT PLACE OR OPERATE RIDING TROWELS ON FRAMED FLOORS.



1 S2.1 First Floor Framing Plan
1/8" = 1'-0"

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SHEET TITLE
FIRST FLOOR FRAMING PLAN

Date:
10-01-2018

PROJECT NUMBER
18-115

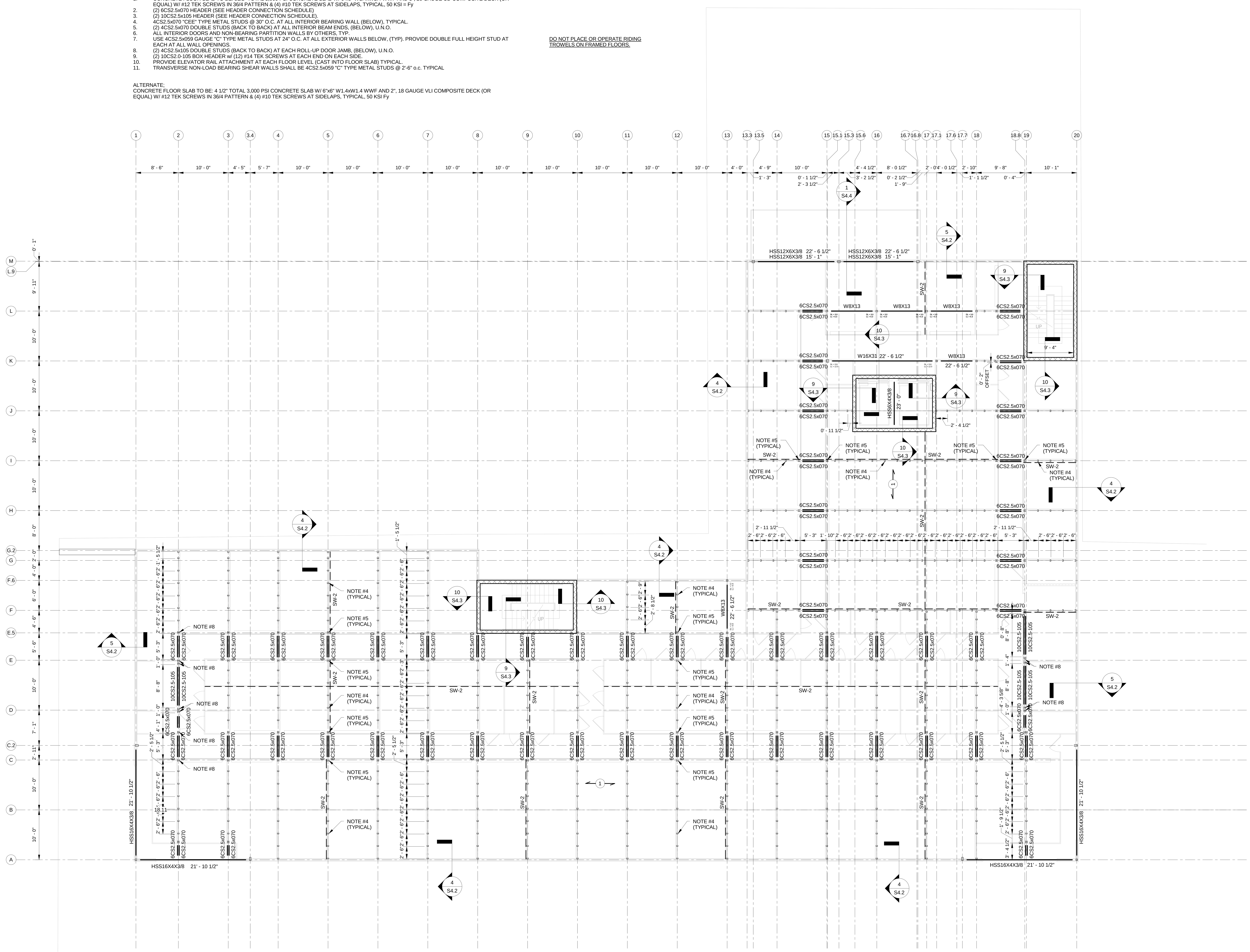
SHEET NUMBER
S2.1

ISSUED FOR CONSTRUCTION

1. CONCRETE FLOOR SLAB TO BE: 51/2" TOTAL 3,000 PSI CONCRETE SLAB W/ 6"x6" W1.4xW1.4 WWF AND 3", 20 GAUGE CD COMPOSITE DECK (OR EQUAL) W/ #12 TEK SCREWS IN 36/4 PATTERN & (4) #10 TEK SCREWS AT SIDELAPS, TYPICAL, 50 KSI = Fy
2. (2) 6CS2.5x070 HEADER (SEE HEADER CONNECTION SCHEDULE)
3. (2) 10CS2.5x105 HEADER (SEE HEADER CONNECTION SCHEDULE).
4. 4CS2.5x070 "CEE" TYPE METAL STUDS @ 30" O.C. AT ALL INTERIOR BEARING WALL (BELOW), TYPICAL.
5. (2) 4CS2.5x070 DOUBLE STUDS (BACK TO BACK) AT ALL INTERIOR BEAM ENDS, (BELOW), U.N.O.
6. ALL INTERIOR DOORS AND NON-BEARING PARTITION WALLS BY OTHERS, TYP.
7. USE 4CS2.5x059 GAUGE "C" TYPE METAL STUDS AT 24" O.C. AT ALL EXTERIOR WALLS BELOW, (TYP). PROVIDE DOUBLE FULL HEIGHT STUD AT EACH AT ALL WALL OPENINGS.
8. (2) 4CS2.5x105 DOUBLE STUDS (BACK TO BACK) AT EACH ROLL-UP DOOR JAMB, (BELOW), U.N.O.
9. (2) 10CS2.0-105 BOX HEADER w/ (12) #14 TEK SCREWS AT EACH END ON EACH SIDE.
10. PROVIDE ELEVATOR RAIL ATTACHMENT AT EACH FLOOR LEVEL (CAST INTO FLOOR SLAB) TYPICAL.
11. TRANSVERSE NON-LOAD BEARING SHEAR WALLS SHALL BE 4CS2.5x059 "C" TYPE METAL STUDS @ 2'-6" o.c. TYPICAL

DO NOT PLACE OR OPERATE RIDING TROWELS ON FRAMED FLOORS.

ALTERNATE;
CONCRETE FLOOR SLAB TO BE: 4 1/2" TOTAL 3,000 PSI CONCRETE SLAB W/ 6"x6" W1.4xW1.4 WWF AND 2", 18 GAUGE VLI COMPOSITE DECK (OR EQUAL) W/ #12 TEK SCREWS IN 36/4 PATTERN & (4) #10 TEK SCREWS AT SIDELAPS, TYPICAL, 50 KSI Fy



1 S2.2 Second Floor Framing Plan
1/8" = 1'-0"

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SHEET TITLE
SECOND FLOOR FRAMING PLAN

Date:
10-01-2018

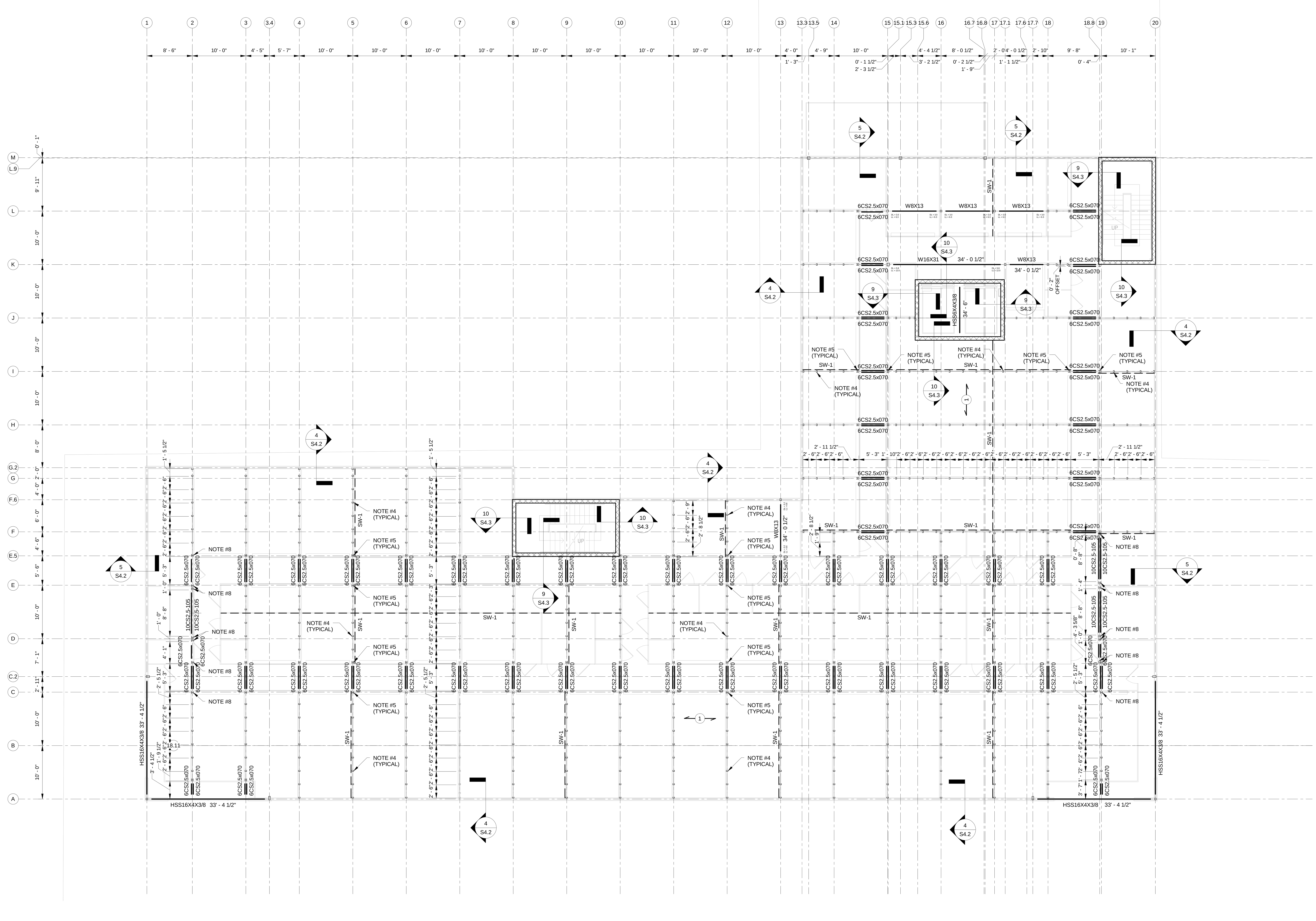
PROJECT NUMBER
18-115

SHEET NUMBER
S2.2

1. CONCRETE FLOOR SLAB TO BE: 51/2" TOTAL 3,000 PSI CONCRETE SLAB W/ 6"x6" W1.4xW1.4 WWF AND 3", 20 GAUGE COMPOSITE DECK (OR EQUAL) W/ #12 TEK SCREWS IN 36/4 PATTERN & (4) #10 TEK SCREWS AT SIDELAPS, TYPICAL, 50 KSI = Fy
2. (2) 6CS2.5x070 HEADER (SEE HEADER CONNECTION SCHEDULE).
3. (2) 10CS2.5x105 HEADER (SEE HEADER CONNECTION SCHEDULE).
4. 4CS2.5x059 "CEE" TYPE METAL STUDS @ 30" O.C. AT ALL INTERIOR BEARING WALLS (BELOW), TYPICAL.
5. (2) 4CS2.5x059 DOUBLE STUDS (BACK TO BACK) AT ALL INTERIOR BEAM ENDS, (BELOW), U.N.O. PROVIDE DOUBLE "CEE" STUDS (BACK TO BACK) FROM THIS FRAMING LEVEL TO FOUNDATION BELOW, UNDER ALL DOUBLE "CEE" STUDS AT THIS LEVEL.
6. ALL INTERIOR DOORS AND NON-BEARING PARTITION WALLS BY OTHERS, TYP.
7. USE 4CS2.5x059 GAUGE "C" TYPE METAL STUDS AT 24" O.C. AT ALL EXTERIOR WALLS BELOW, (TYP). PROVIDE DOUBLE FULL HEIGHT STUD AT EACH AT ALL WALL OPENINGS.
8. (2) 4CS2.5x070 DOUBLE STUDS (BACK TO BACK) AT EACH ROLL-UP DOOR JAMB, (BELOW), U.N.O. PROVIDE DOUBLE "CEE" STUDS (BACK TO BACK) FROM THIS FRAMING LEVEL TO FOUNDATION BELOW, UNDER ALL DOUBLE "CEE" STUDS AT THIS LEVEL.
9. (2) 8CS2.0-105 BOX HEADER w/ (12) #14 TEK SCREWS AT EACH END ON EACH SIDE.
10. PROVIDE ELEVATOR RAIL ATTACHMENT AT EACH FLOOR LEVEL (CAST INTO FLOOR SLAB) TYPICAL.
11. TRANSVERSE NON-LOAD BEARING SHEAR WALLS SHALL BE 4CS2.5x059 "C" TYPE METAL STUDS @ 2'-6" o.c. TYPICAL

ALTERNATE;
CONCRETE FLOOR SLAB TO BE: 4 1/2" TOTAL 3,000 PSI CONCRETE SLAB W/ 6"x6" W1.4xW1.4 WWF AND 2", 18 GAUGE VLI COMPOSITE DECK (OR EQUAL) W/ #12 TEK SCREWS IN 36/4 PATTERN & (4) #10 TEK SCREWS AT SIDELAPS, TYPICAL, 50 KSI Fy

DO NOT PLACE OR OPERATE RIDING TROWELS ON FRAMED FLOORS.



1 S2.3 Third Floor Framing Plan
1/8" = 1'-0"

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SHEET TITLE
THIRD FLOOR FRAMING PLAN

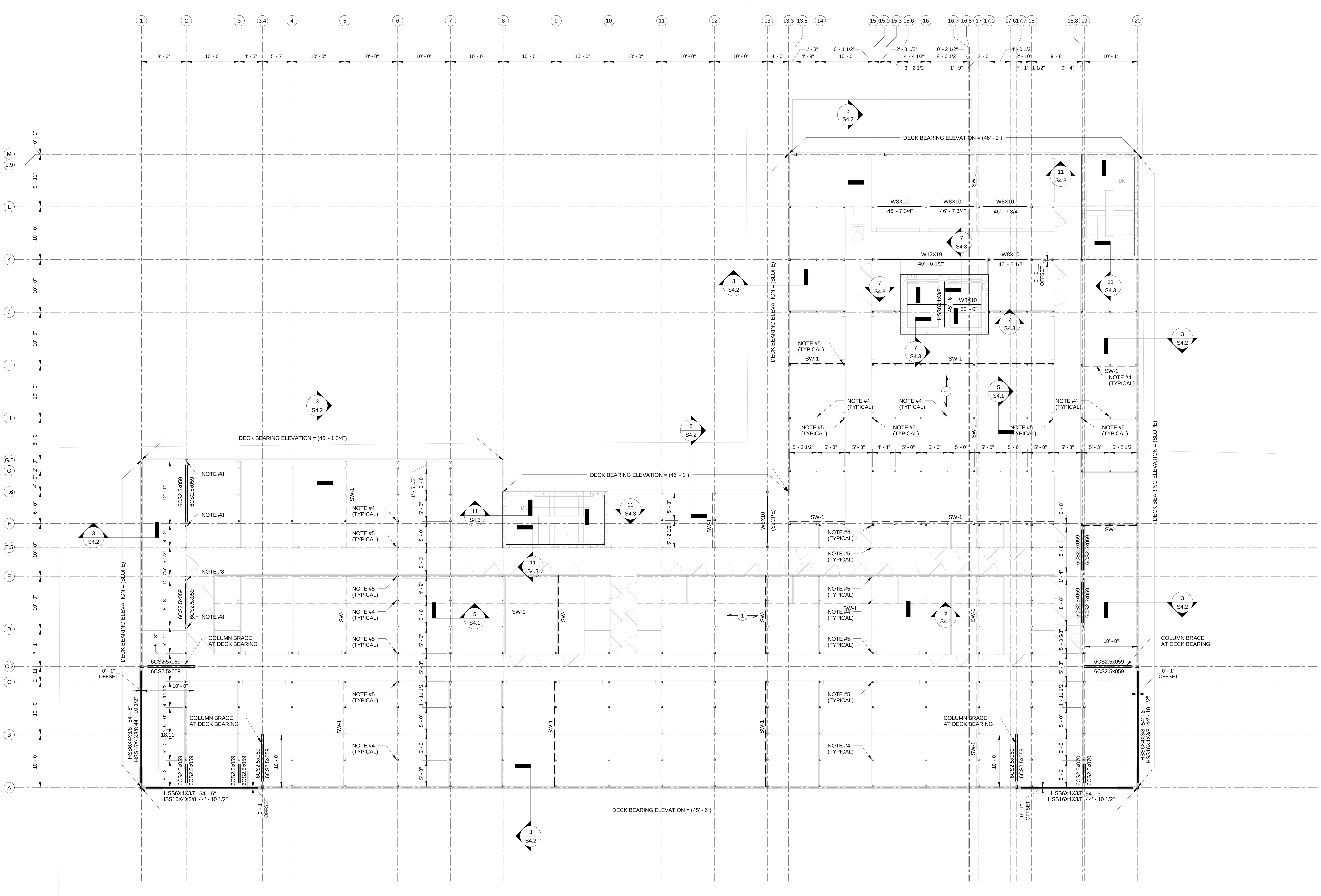
Date:
10-01-2018

PROJECT NUMBER
18-115

SHEET NUMBER
S2.3

ISSUED FOR CONSTRUCTION

1. ROOF DECK TO BE 3", TYPE "N", 22 GA. W/ #12 TEK SCREWS IN 24/4 PATTERN & (7) #10 TEK SCREWS AT SIDELAPS, TYP.
2. (2) 4CS2.5x059 HEADER (SEE HEADER CONNECTION SCHEDULE)
3. (2) 6CS2.5x059 HEADER (SEE HEADER CONNECTION SCHEDULE).
4. 4CS2.5x059 "CEE" TYPE METAL STUDS @ 5'-0" O.C. AT ALL INTERIOR BEARING WALLS U.N.O. (BELOW), TYPICAL.
5. PROVIDE SINGLE 4CS2.5x059 "CEE" TYPE STUD AT EACH ROLLUP DOOR JAMB AND AT EACH SIDE OF CORRIDORS FROM THIS FRAMING LEVEL TO THE FOUNDATION BELOW.
6. ALL INTERIOR DOORS AND NON-BEARING PARTITION WALLS BY OTHERS, TYP.
7. USE 4CS2.5x059 GAUGE "C" TYPE METAL STUDS AT 24" O.C. AT ALL EXTERIOR WALLS BELOW, (TYP). PROVIDE DOUBLE FULL HEIGHT STUD AT EACH AT ALL WALL OPENINGS.
8. (2) 4CS2.0x059 DOUBLE STUDS (BACK TO BACK) AT EACH ROLL-UP DOOR JAMB, U.N.O. UNDER ALL DOUBLE "CEE" STUDS AT THIS LEVEL, PROVIDE DOUBLE "CEE" STUDS (BACK TO BACK) TO FOUNDATION BELOW.
9. 6CS2.5x059 "CEE" TYPE METAL STUDS @ 5'-0" O.C. AT ALL INTERIOR BEARING WALLS U.N.O. (BELOW), TYPICAL.
10. (2) 10CS2.0-105 BOX HEADER w/ 4T3.0-105 TOP AND BOTTOM TRACK
11. (2) 8CS2.0-105 BOX HEADER w/ (8) #14 TEK SCREWS AT EACH END ON EACH SIDE.
12. TRANSVERSE NON-LOAD BEARING SHEAR WALLS SHALL BE 4CS2.5x059 "C" TYPE METAL STUDS @ 5'-0" o.c. TYPICAL
13. PROVIDE SINGLE 6CS2.5x059 "CEE" TYPE STUD AT EACH ROLLUP DOOR JAMB AND AT EACH SIDE OF CORRIDORS FROM THIS FRAMING LEVEL TO THE FOUNDATION BELOW.



1 S2.4 Roof Framing Plan
1/8" = 1'-0"

ISSUED FOR CONSTRUCTION

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GEORGIA REGISTERED PROFESSIONAL ENGINEER No. 38787 BRENDAN F. CROWLEY

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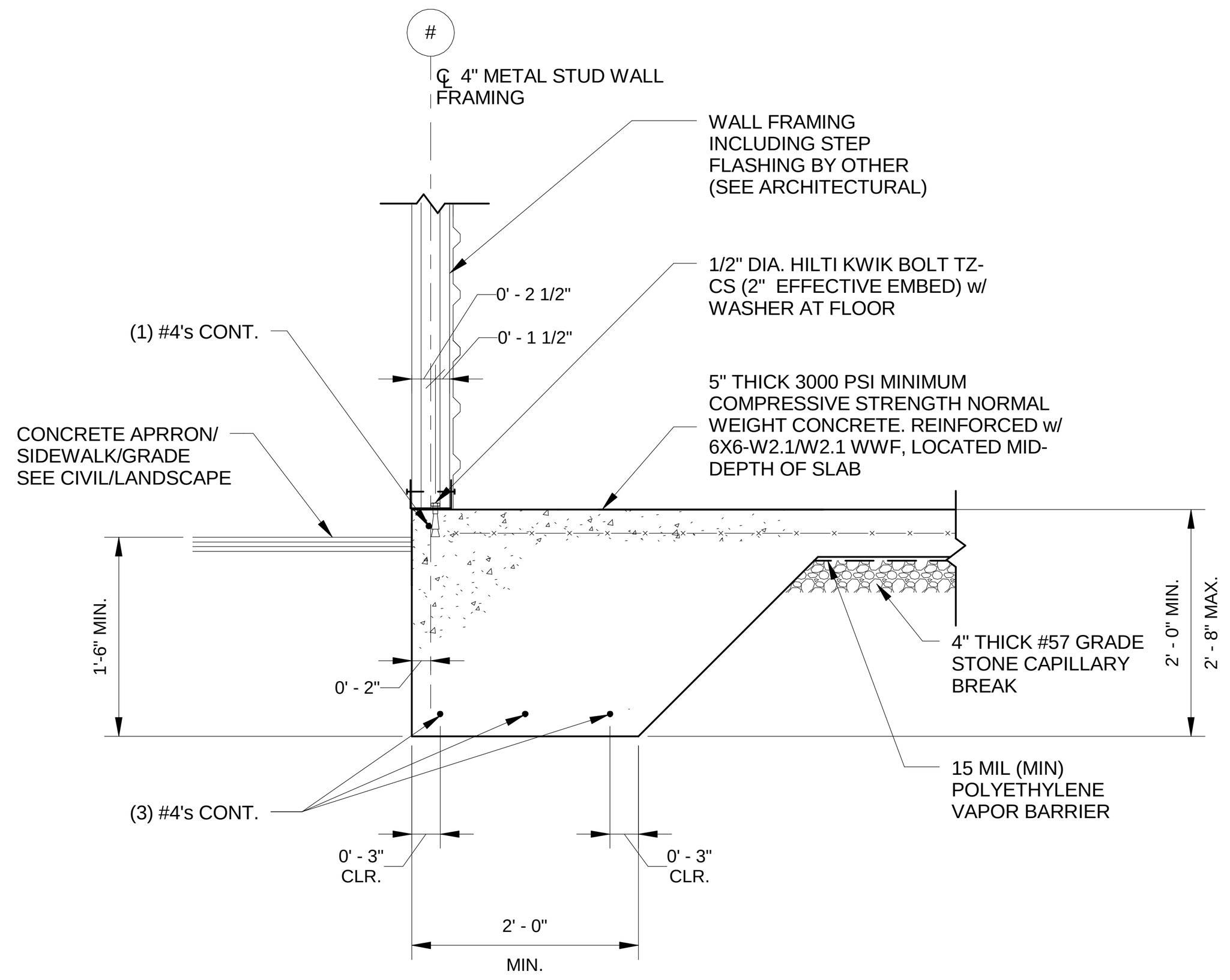
SHEET TITLE
ROOF FRAMING PLAN

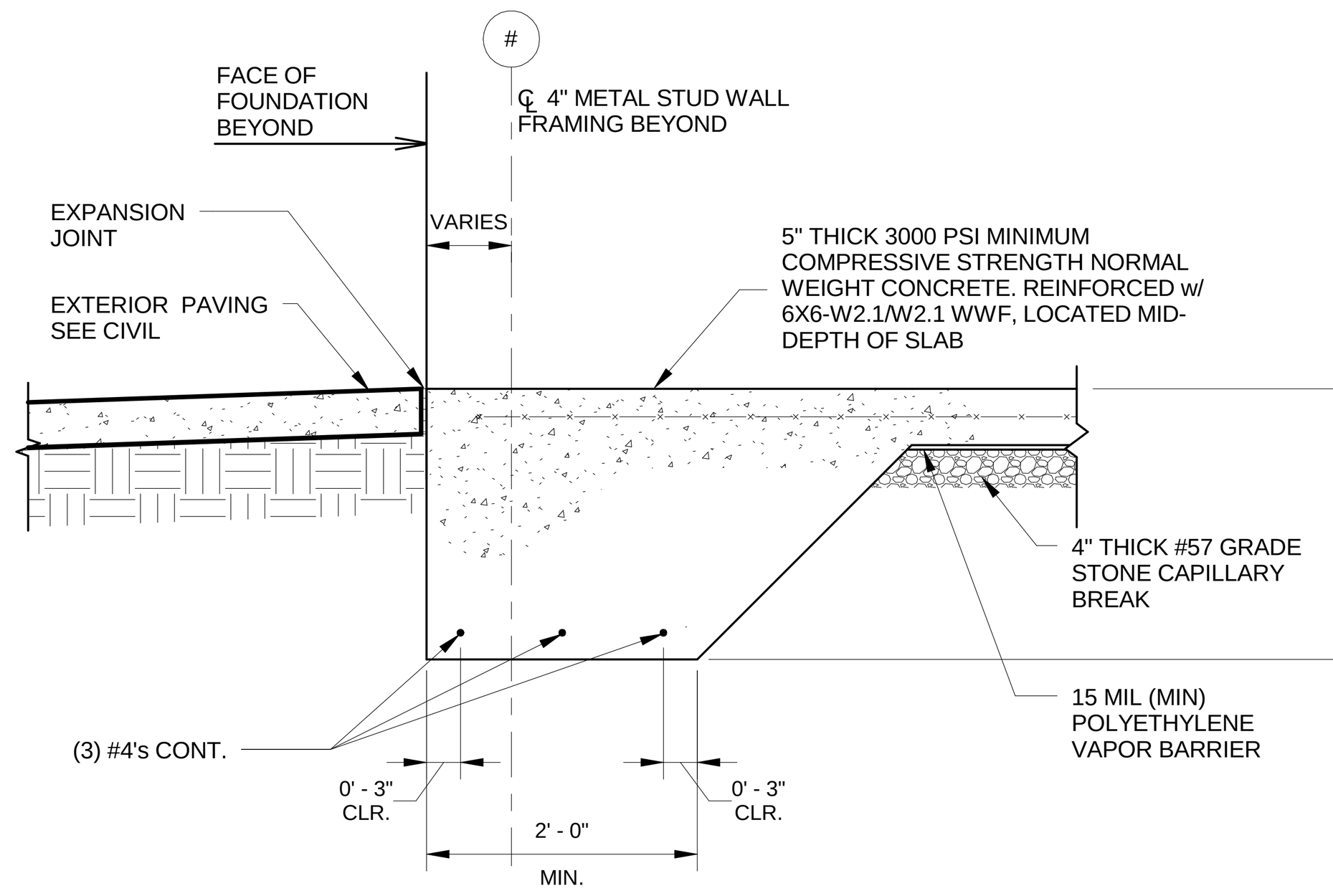
Date:
10-01-2018

PROJECT NUMBER
18-115

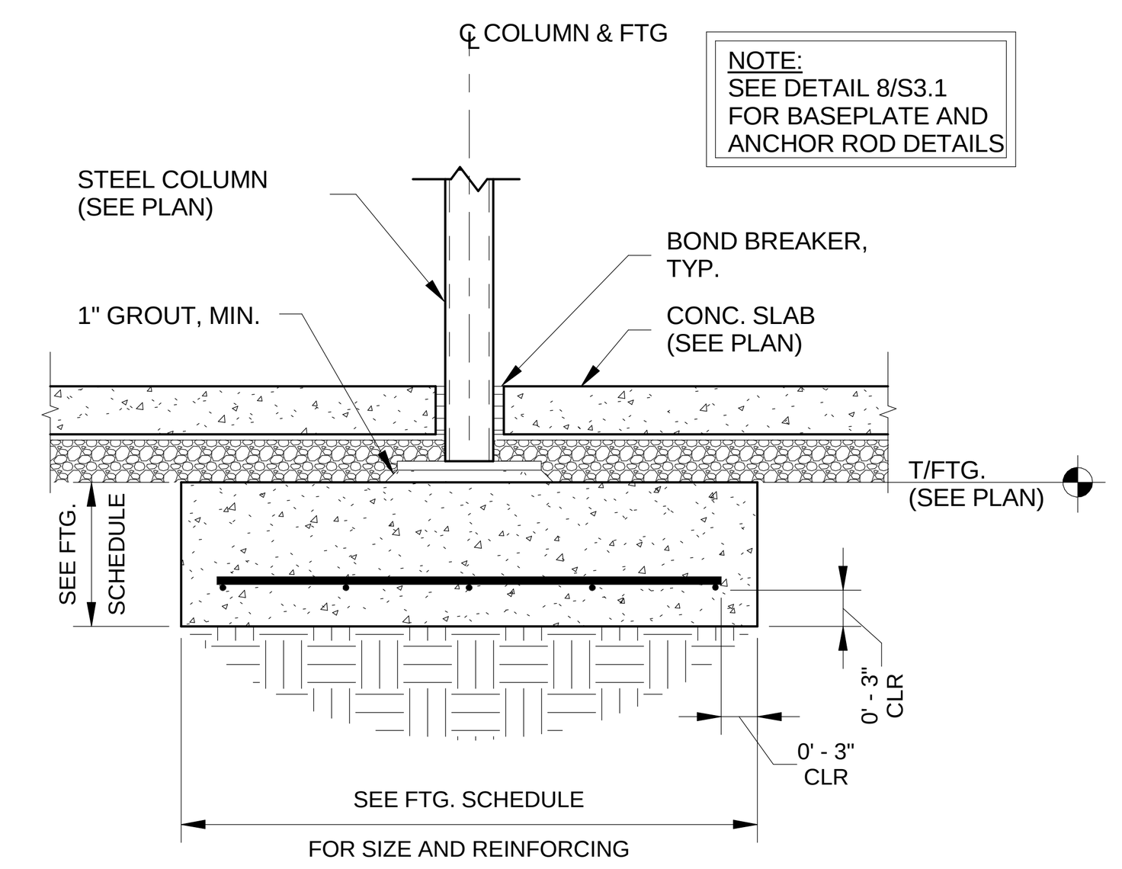
SHEET NUMBER
S2.4



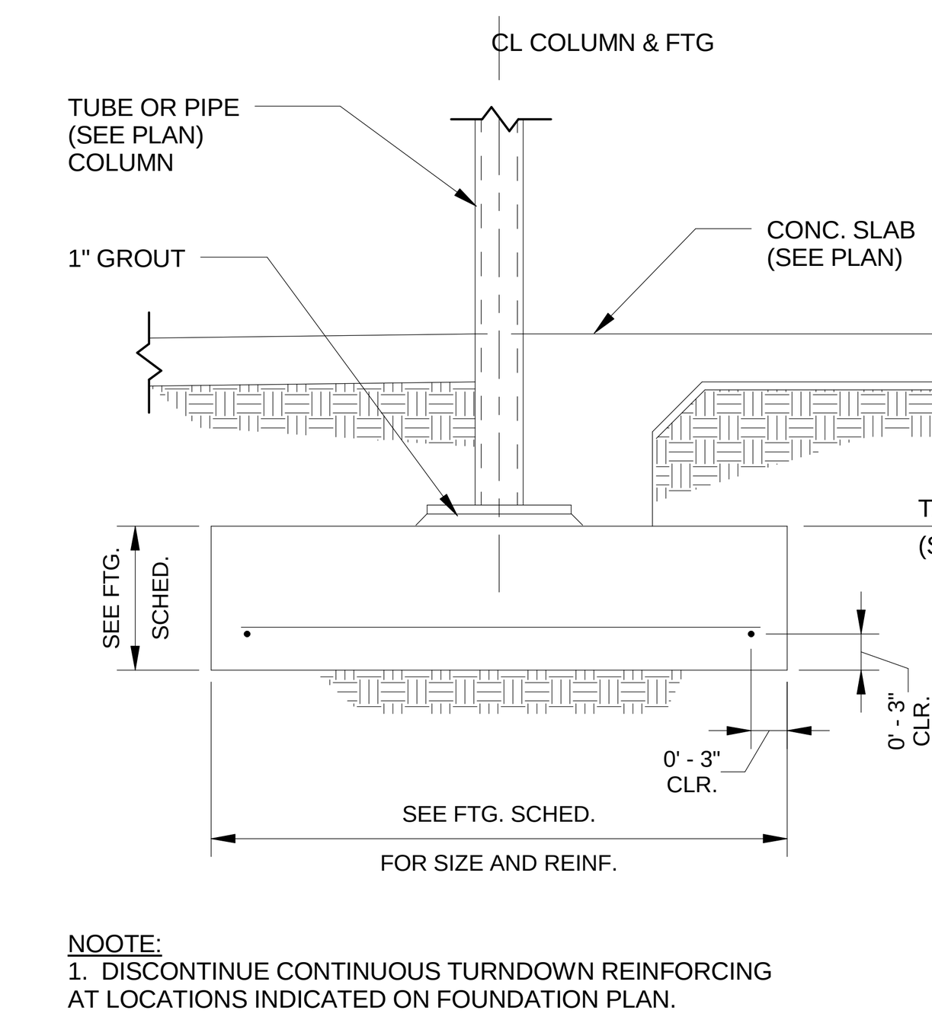
1 S3.1 FOOTING/WALL SECTION
3/4" = 1'-0"



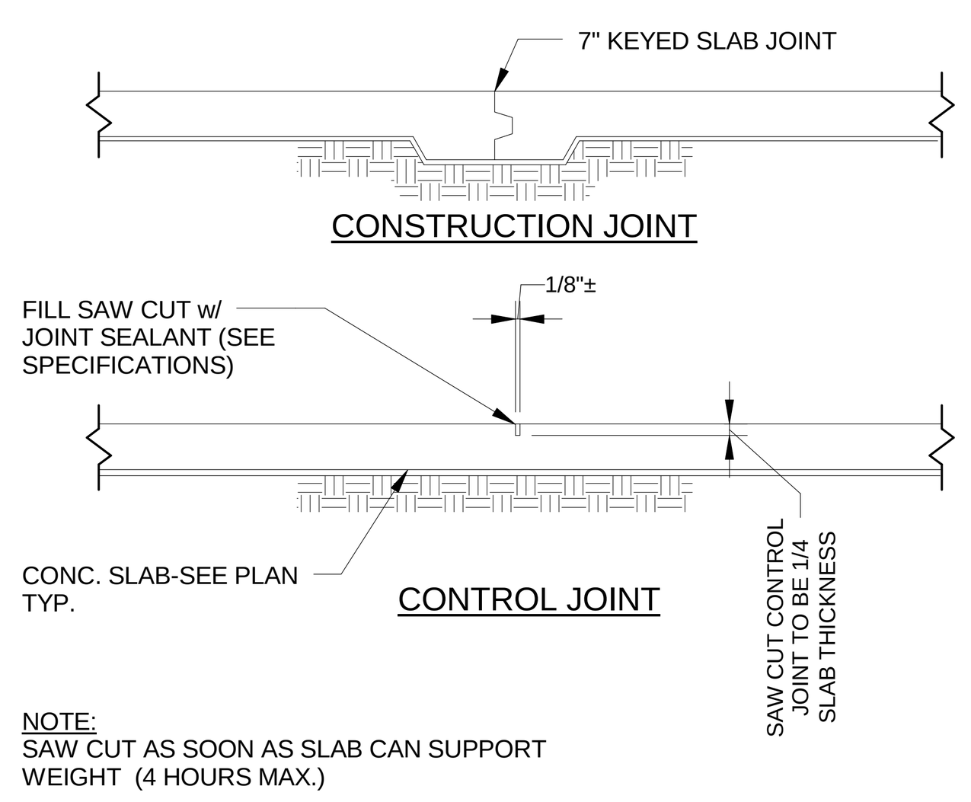
2 S3.1 TURNDOWN FOOTING SECTION
3/4" = 1'-0"



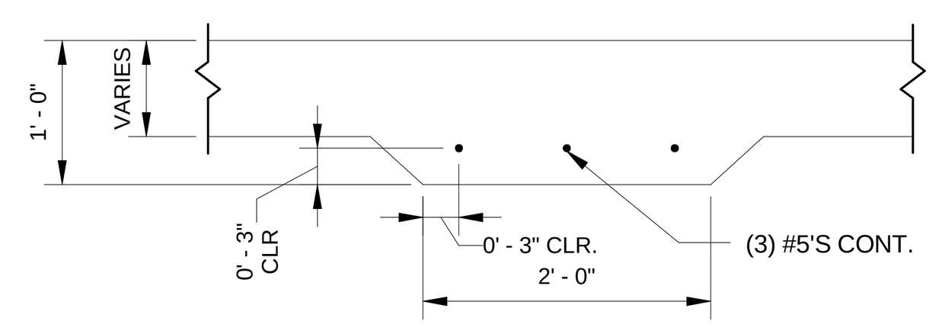
3 S3.1 COLUMN FOOTING DETAIL
3/4" = 1'-0"



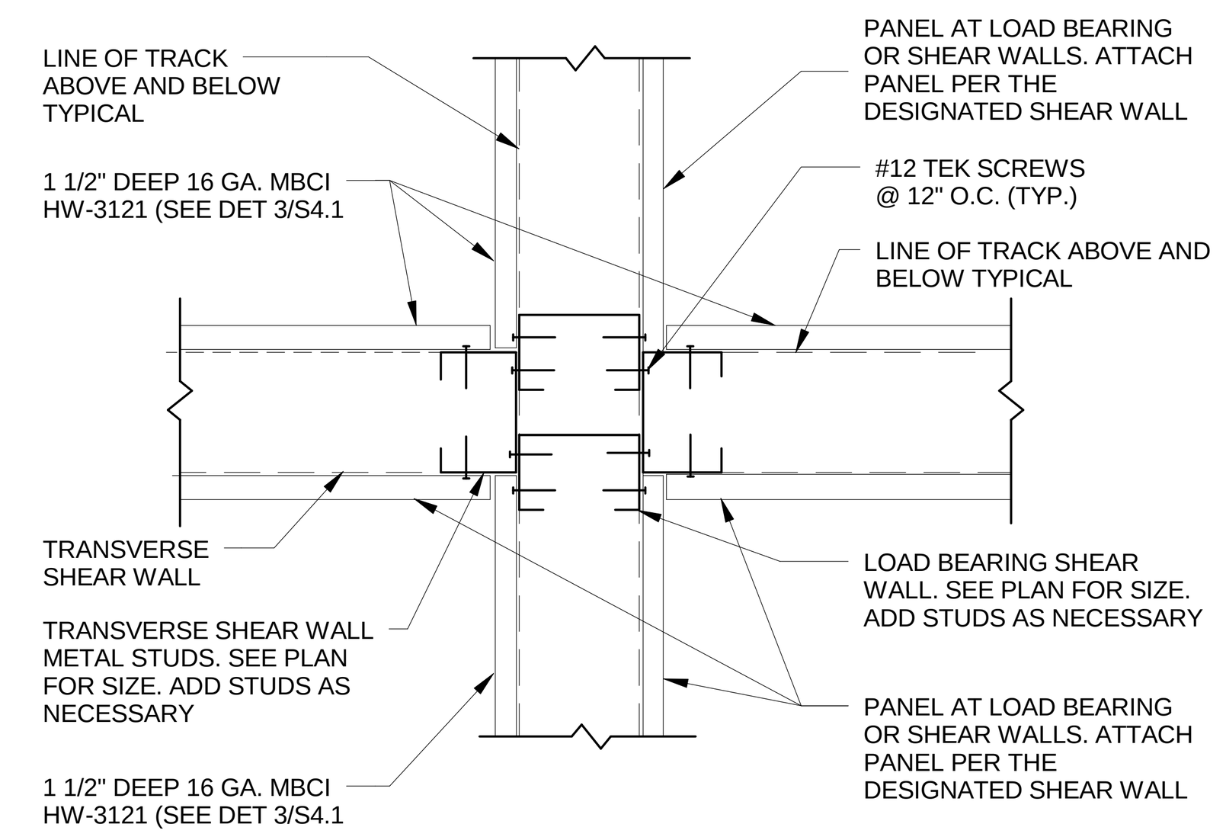
4 S3.1 COLUMN FTG AT STOREFRONT
3/4" = 1'-0"



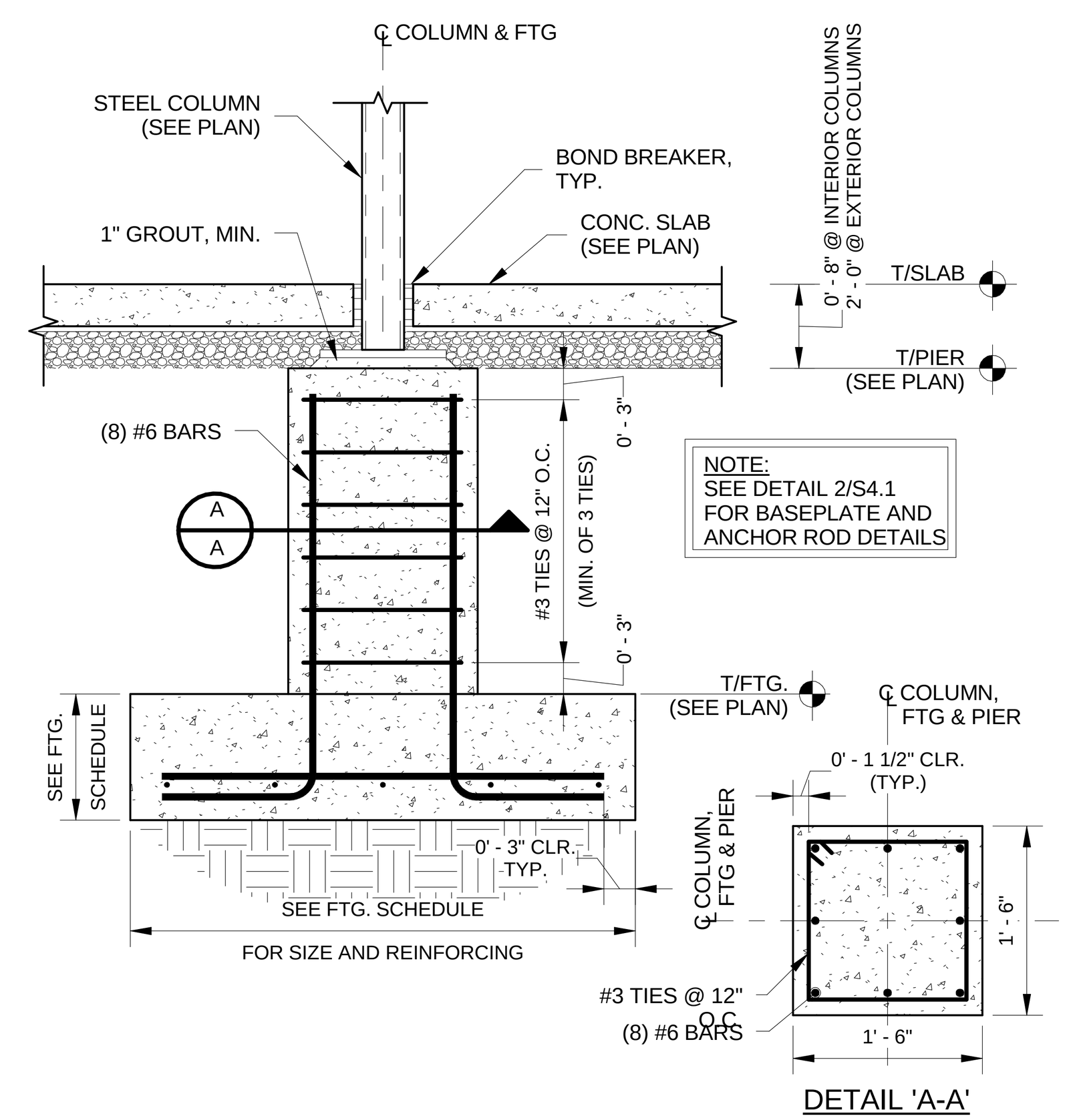
5 S3.1 CONSTRUCTION/CONTROL JOINT DETAILS
3/4" = 1'-0"



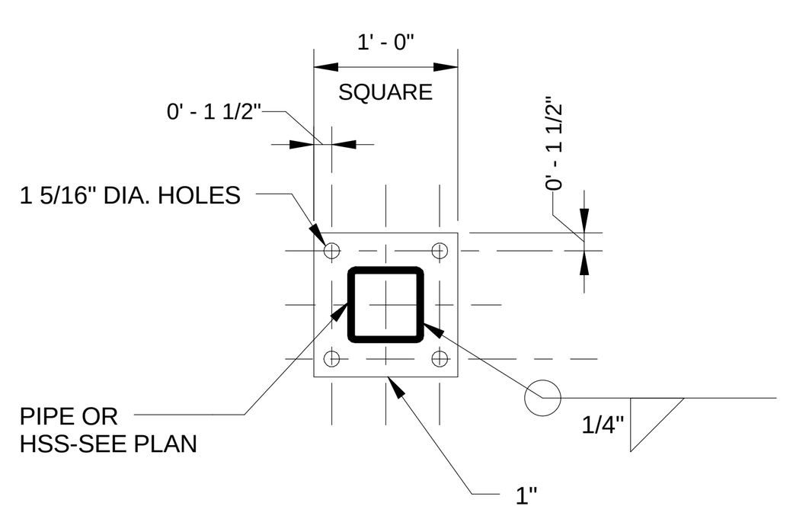
6 S3.1 THICKENED SLAB
3/4" = 1'-0"



7 S3.1 INTERSECTING METAL STUD SHEAR WALL DETAIL
3/4" = 1'-0"

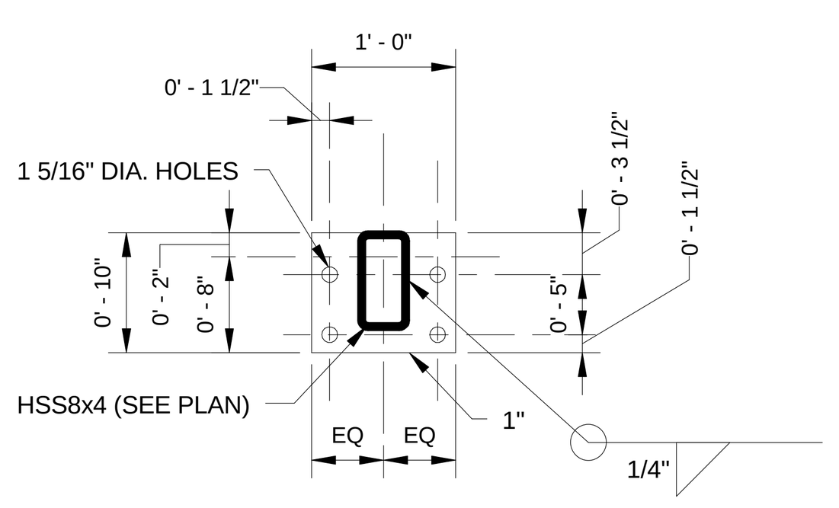


9 S3.1 TYP. CONCRETE PIER DETAIL
3/4" = 1'-0"



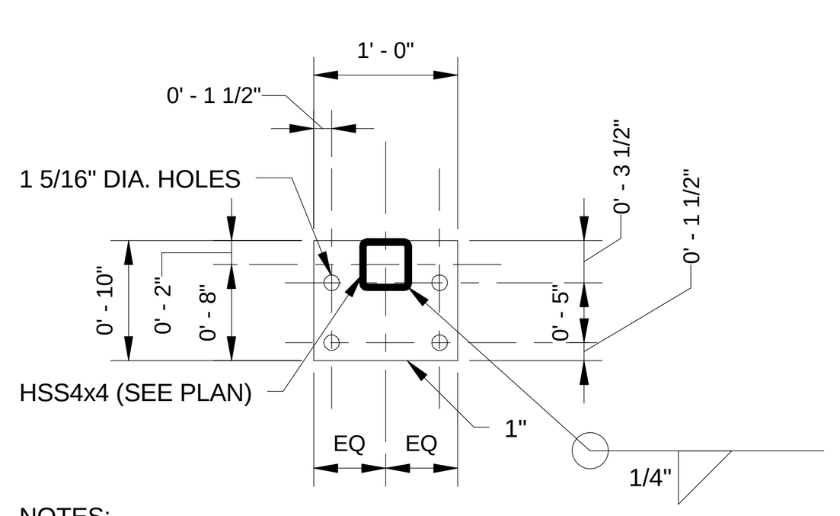
NOTES:
1. COLUMN TO BE CENTERED ON BASE PLATE (TYP. U.N.O.)

BP-1



NOTES:
1. COLUMN NOT CENTERED ON BASE PLATE (TYP. U.N.O.)

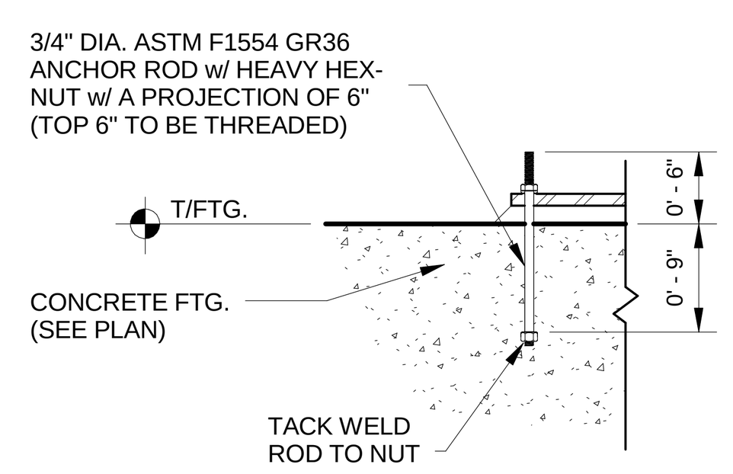
BP-2



NOTES:
1. COLUMN NOT CENTERED ON BASE PLATE (TYP. U.N.O.)

BP-3

8 S3.1 BASE PLATE DETAIL
3/4" = 1'-0"



ANCHOR ROD 'A'

NOTES:
1. ★ DESIGNATES THAT WELD SIZE SHOULD MATCH COLUMN WALL THICKNESS MINUS 1/16" FOR WALL, THICKNESS GREATER THAN 1/4". AT COLUMNS WITH WALL THICKNESSES 1/4" OR LESS, WELD SIZE SHOULD MATCH COLUMN WALL THICKNESS.

2. ANCHOR BOLT PROJECTION TO BE 6" (TYP. - U.N.O.) - (SEE ANCHOR ROD 'A')

LIGHT-GAUGE CEE MEMBER SCHEDULE (MINIMUM PROPERTIES) 1,2

MEMBER DESIGNATION	D (IN.)	B (IN.)	d (IN.)	GAUGE	t (IN.)	AREA (IN.^2)	Ix MOMENT OF INERTIA (IN.^4)	Iy MOMENT OF INERTIA (IN.^4)
4C2x070	4"	2"	0.799	14	0.07	0.626	1.564	0.371
4C2x059	4"	2"	0.773	16	0.059	0.527	1.331	0.314
4C2.5x105	4"	2.5"	0.884	12	0.105	1.043	2.676	0.938
4C2.5x070	4"	2.5"	0.799	14	0.07	0.696	1.835	0.630
4C2.5x059	4"	2.5"	0.773	16	0.059	0.586	1.560	0.533
4C3.5x105	4"	3.5"	0.884	12	0.105	1.253	3.473	2.112
4C3.5x070	4"	3.5"	0.799	14	0.07	0.836	2.375	1.414
4C3.5x059	4"	3.5"	0.773	16	0.059	0.704	2.018	1.193
4C4x070	4"	4"	0.8	14	0.07	0.906	2.646	1.957
6C2.5x070	6"	2.5"	0.799	14	0.07	0.836	4.687	0.729
6C2.5x059	6"	2.5"	0.773	16	0.059	0.704	3.971	0.616
6C3x105	6"	3"	0.884	12	0.105	1.358	7.821	1.690
6C3x070	6"	3"	0.799	14	0.07	0.906	5.302	1.130
6C3x059	6"	3"	0.773	16	0.059	0.763	4.492	0.953
8C2x105	8"	2"	0.884	12	0.105	1.358	12.012	0.697
8C2x070	8"	2"	0.799	14	0.07	0.906	8.109	0.468
8C2x059	8"	2"	0.773	16	0.059	0.763	6.861	0.395
8C2.5x105	8"	2.5"	0.884	12	0.105	1.463	13.649	1.196
8C2.5x070	8"	2.5"	0.799	14	0.07	0.976	9.210	0.800
8C2.5x059	8"	2.5"	0.773	16	0.059	0.822	7.791	0.675
10C2x105	10"	2"	0.884	12	0.105	1.568	20.745	0.741
10C2x070	10"	2"	0.799	14	0.07	1.046	13.598	0.497
10C2x059	10"	2"	0.773	16	0.059	0.881	11.798	0.419
10C2.5x105	10"	2.5"	0.884	12	0.105	1.673	23316	1.277
10C2.5x070	10"	2.5"	0.799	14	0.07	1.116	15.684	0.853
10C2.5x059	10"	2.5"	0.773	16	0.059	0.940	13.256	0.719
10C3x105	10"	3"	0.884	12	0.105	1.778	25.886	1.995
10C3x070	10"	3"	0.799	14	0.07	1.186	17.410	1.330
10C3.5x105	10"	3.5"	0.884	12	0.105	1.883	28.456	2.912
10C3.5x070	10"	3.5"	0.799	14	0.07	1.256	19.135	1.939
12C2.5x105	12"	2.5"	0.884	12	0.105	1.883	36.329	1.340
12C2.5x070	12"	2.5"	0.799	14	0.07	1.256	24.390	0.894
12C3x105	12"	3"	0.884	12	0.105	1.988	40.043	2.099
12C3x070	12"	3"	0.799	14	0.07	1.326	26.880	1.398
12C3.5x105	12"	3.5"	0.884	12	0.105	2.093	43.758	3.071
12C4x105	12"	4"	0.884	12	0.105	2.198	47.472	4.275

NOTES:
1. PRIMED STEEL MEMBERS SHALL MEET THE PHYSICAL AND CHEMICAL PROPERTIES OF ASTM A 1011. GRADE 55.
2. ZINC-COATED (GALVANIZED) MEMBERS MEET THE PHYSICAL AND CHEMICAL OF ASTM A 653, GRADE 55 AND G60 COATING DESIGNATION AS DESCRIBED IN ASTM A 924.

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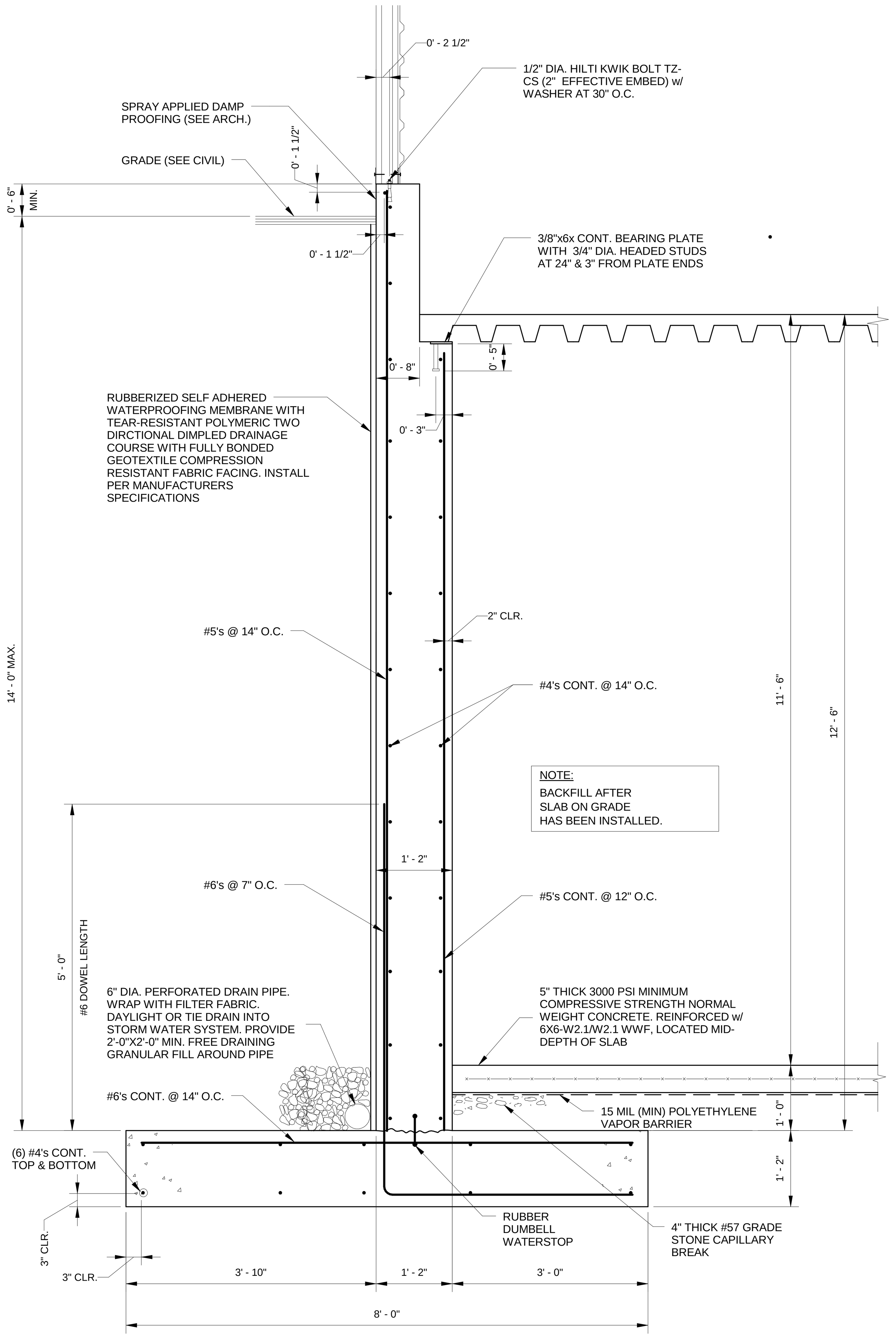
SHEET TITLE
FOUNDATION DETAILS

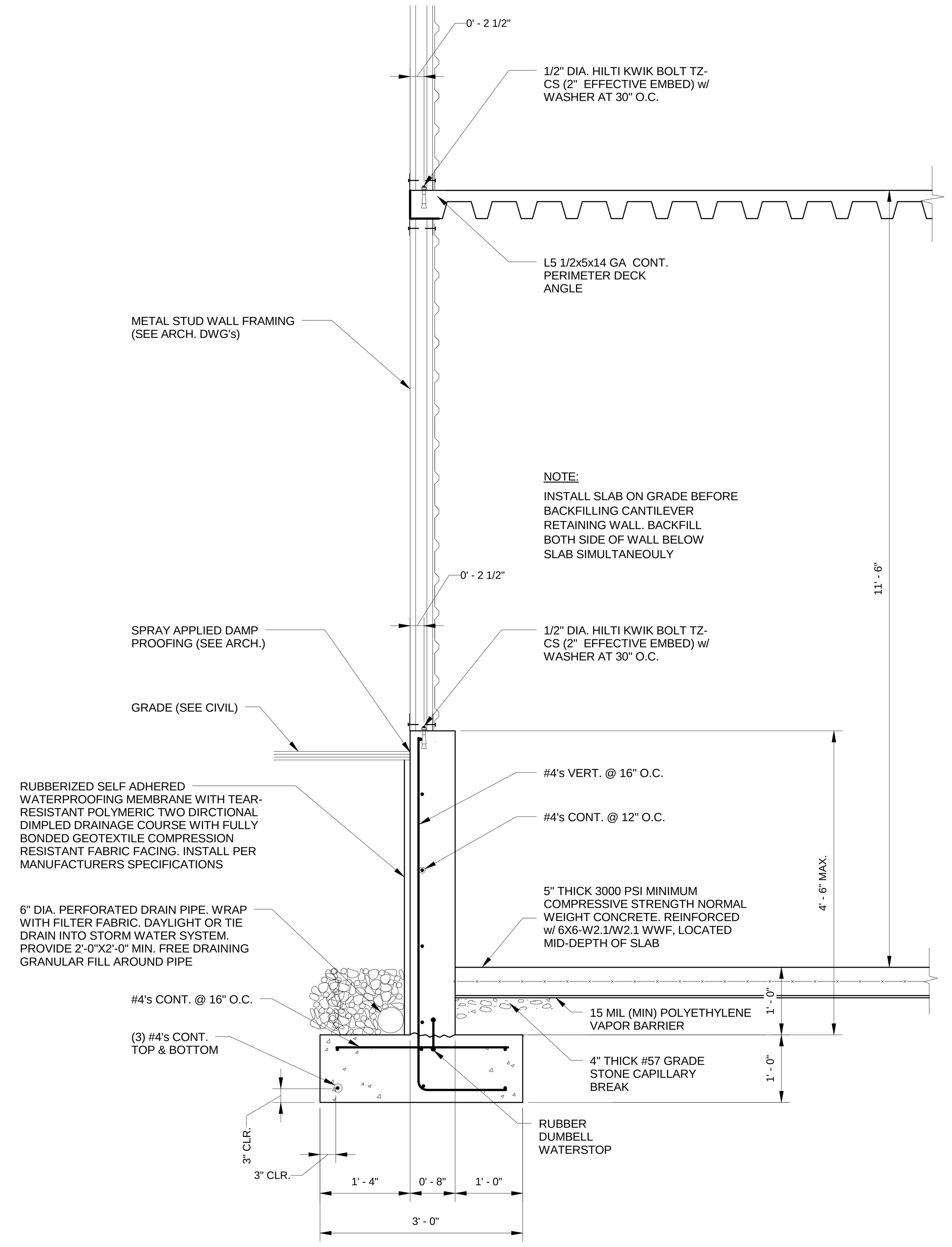
Date:
10-01-2018

PROJECT NUMBER
18-115

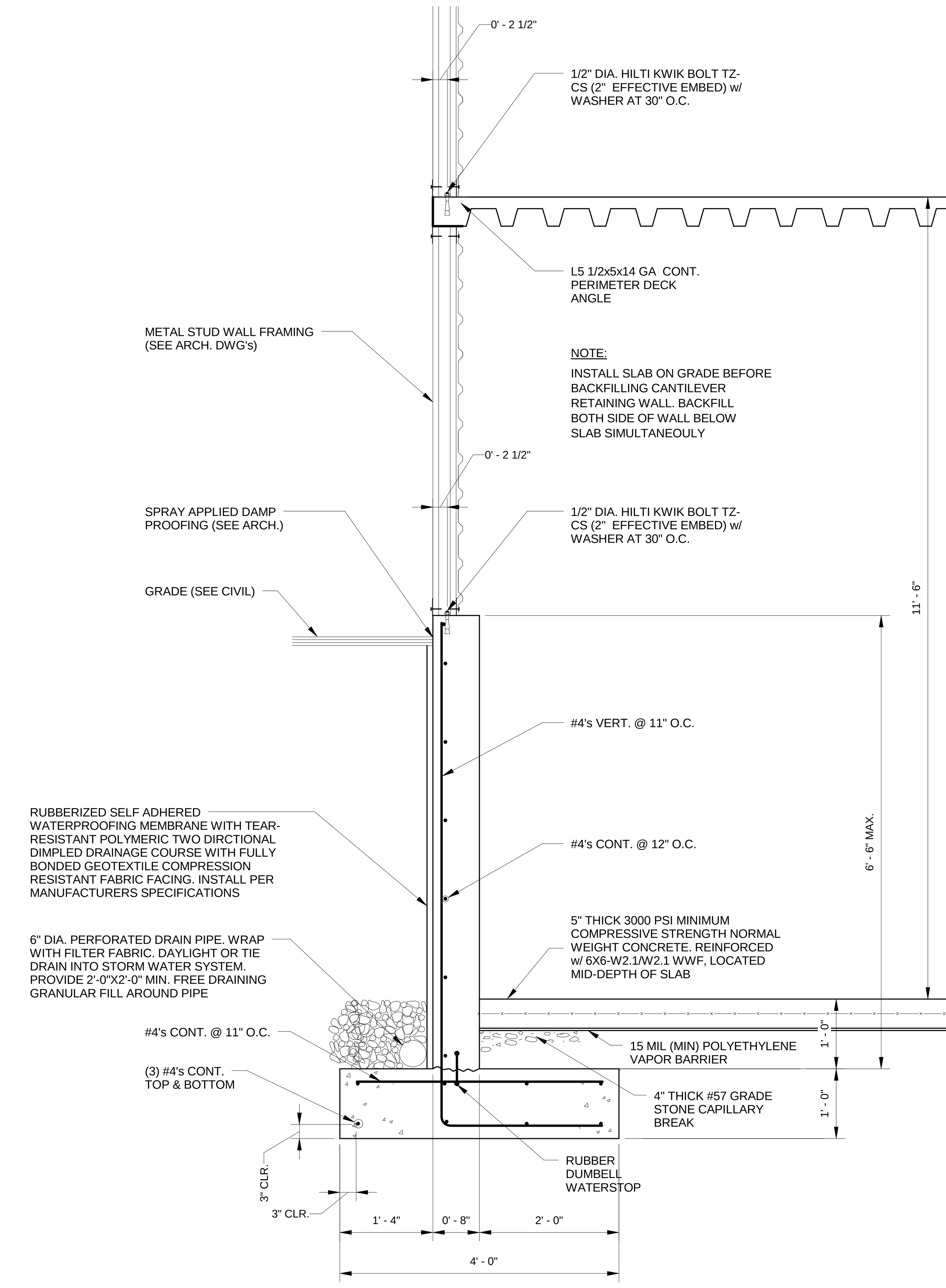
SHEET NUMBER
S3.1



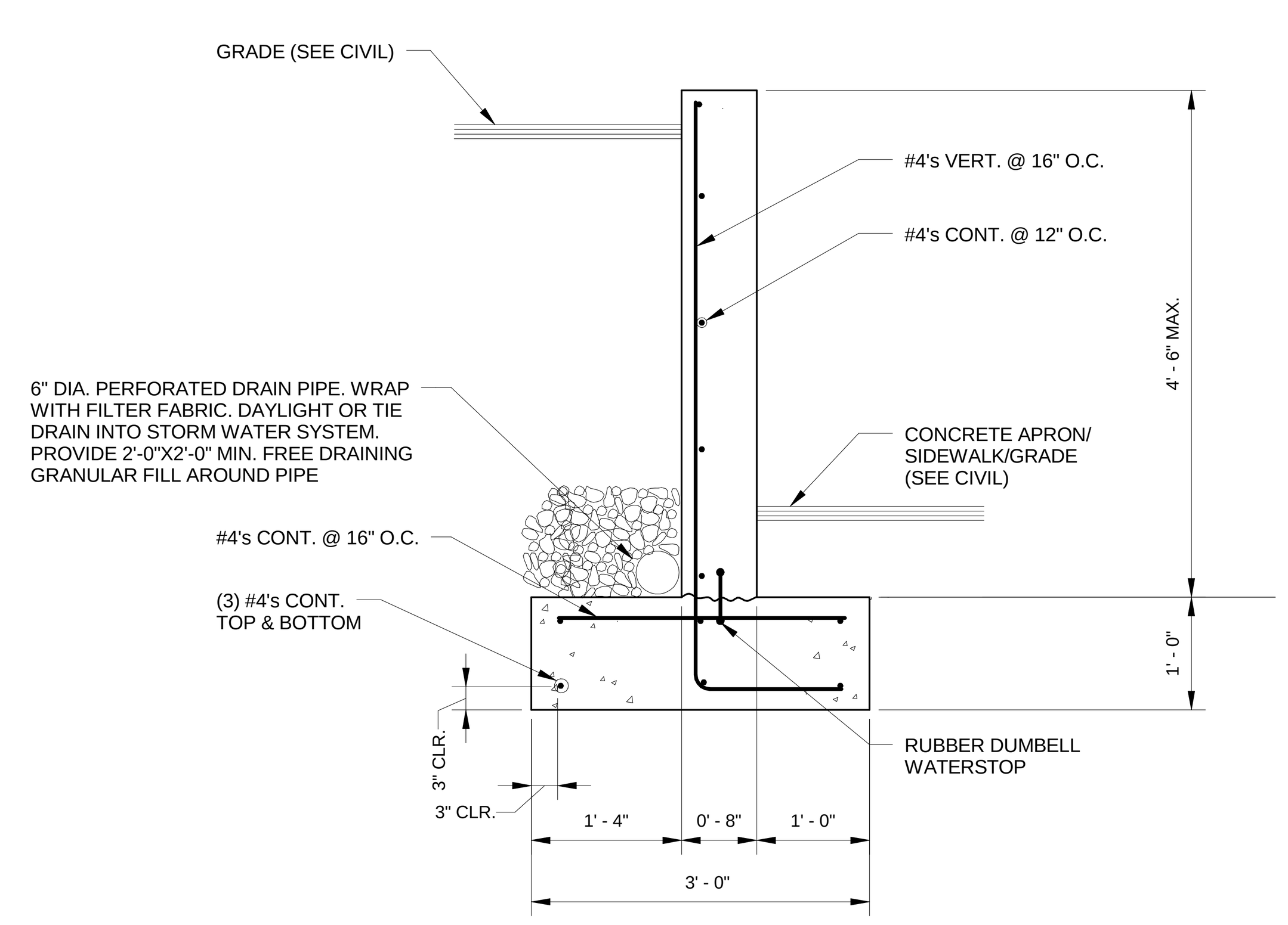
1 S3.2 RETAINING WALL SECTION
3/4" = 1'-0"



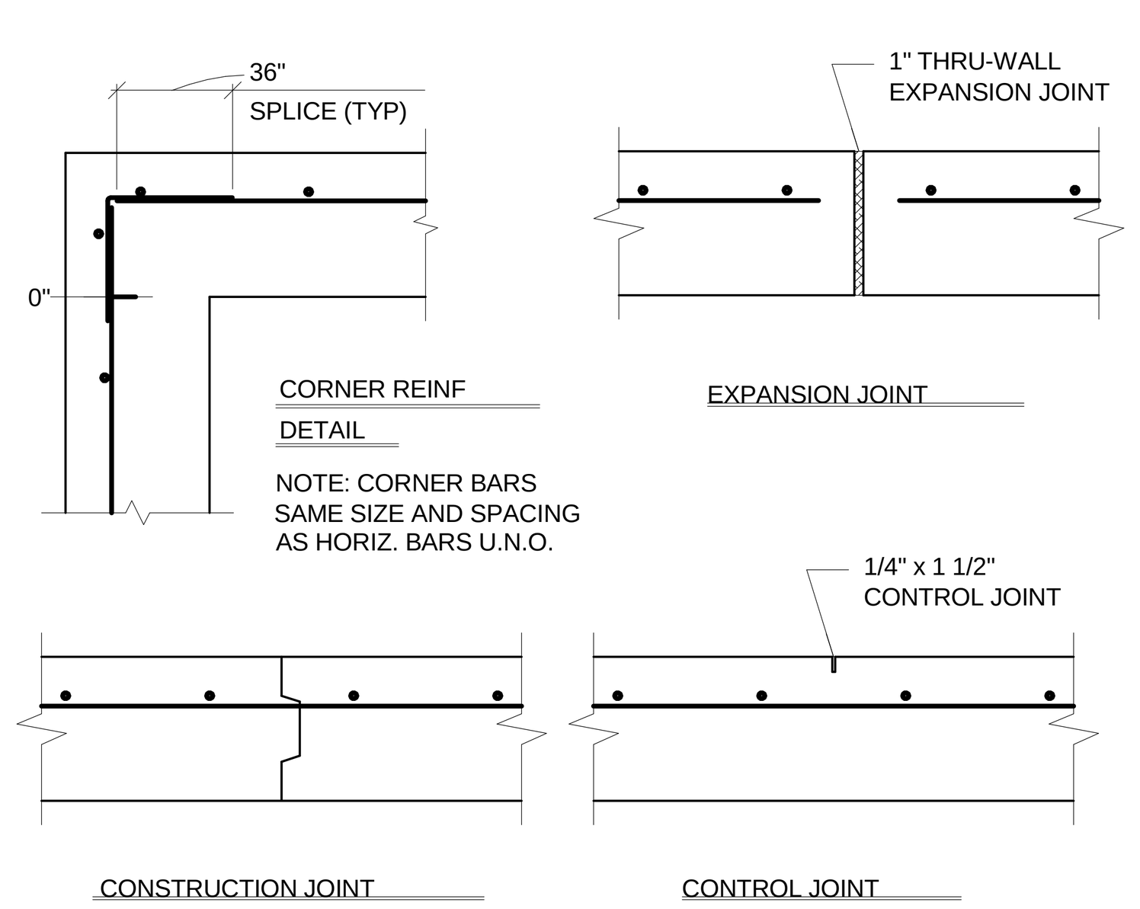
2 S3.2 RETAINING WALL SECTION
3/4" = 1'-0"



3 S3.2 RETAINING WALL SECTION
3/4" = 1'-0"

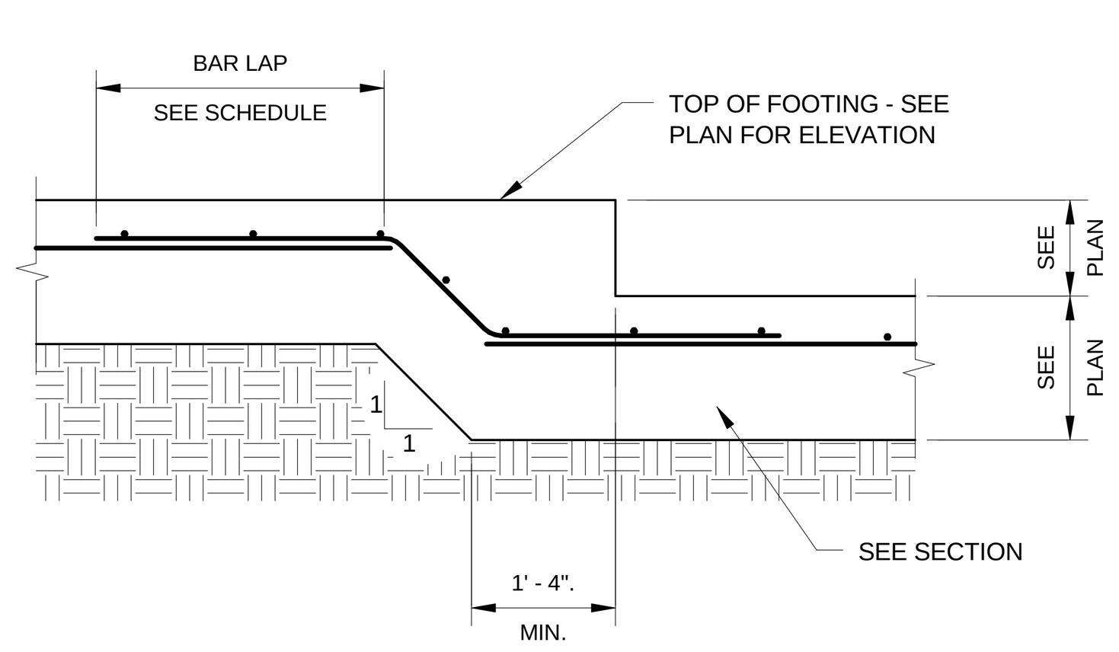


4 S3.2 RETAINING WALL SECTION 4/S3.2
3/4" = 1'-0"



REINFORCING IS SHOWN IN ONE FACE OF WALL.
SEE WALL SECTION AND SCHEDULE FOR EXACT CONDITION.

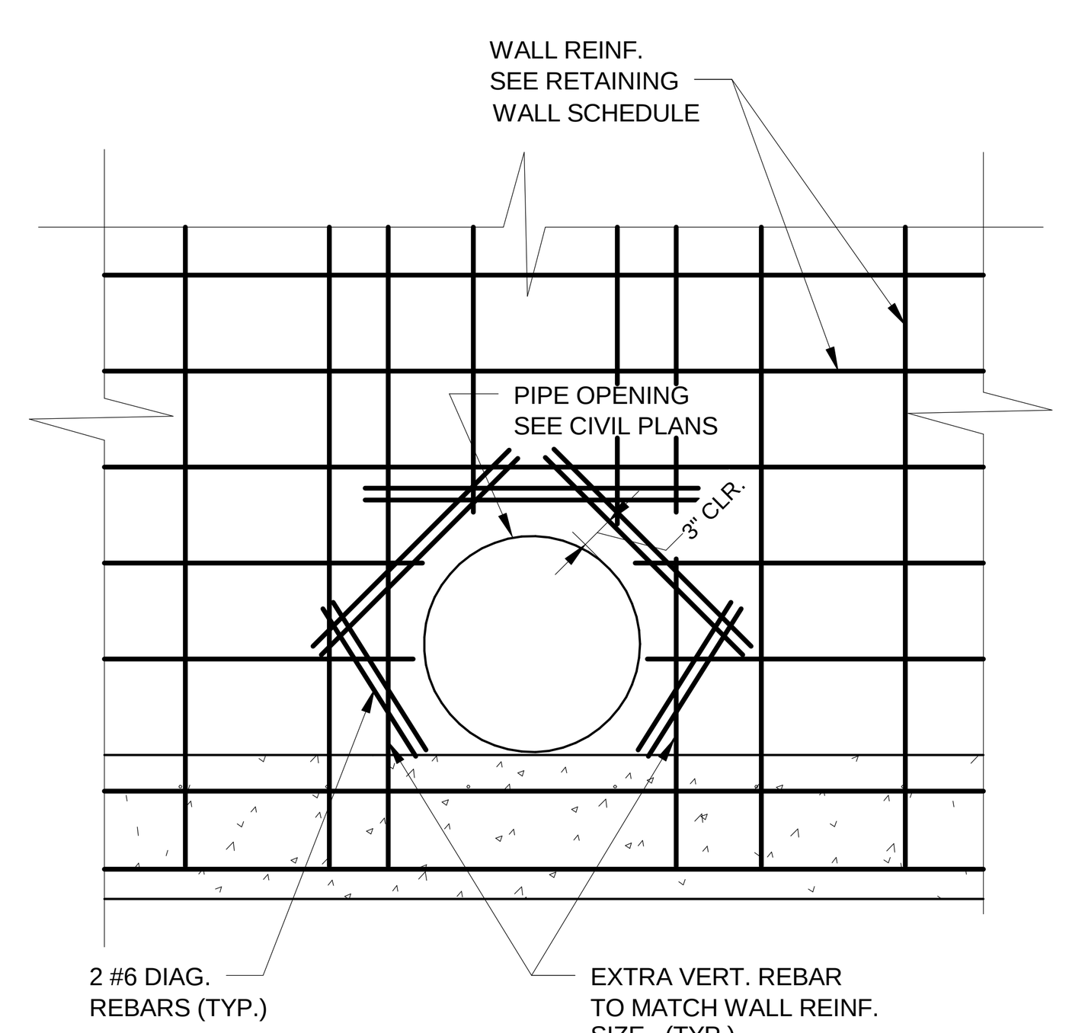
TYPICAL WALL JOINTS & DETAILS



(WALL NOT SHOWN FOR CLARITY)

FOOTING STEP DETAIL

REINFORCED CONCRETE TENSION LAP SPLICE LENGTHS (INCHES) TABLE 1		
BAR SIZE	f`c=3000 PSI	f`c=4000 PSI
#3	25	21.3
#4	33	29
#5	41	36
#6	49	43
#7	72	62



PIPE SHALL NOT INTERFERE WITH BASE OF RETAINING WALL.

REINF. @ PIPE OPENING

RETAINING WALL NOTES

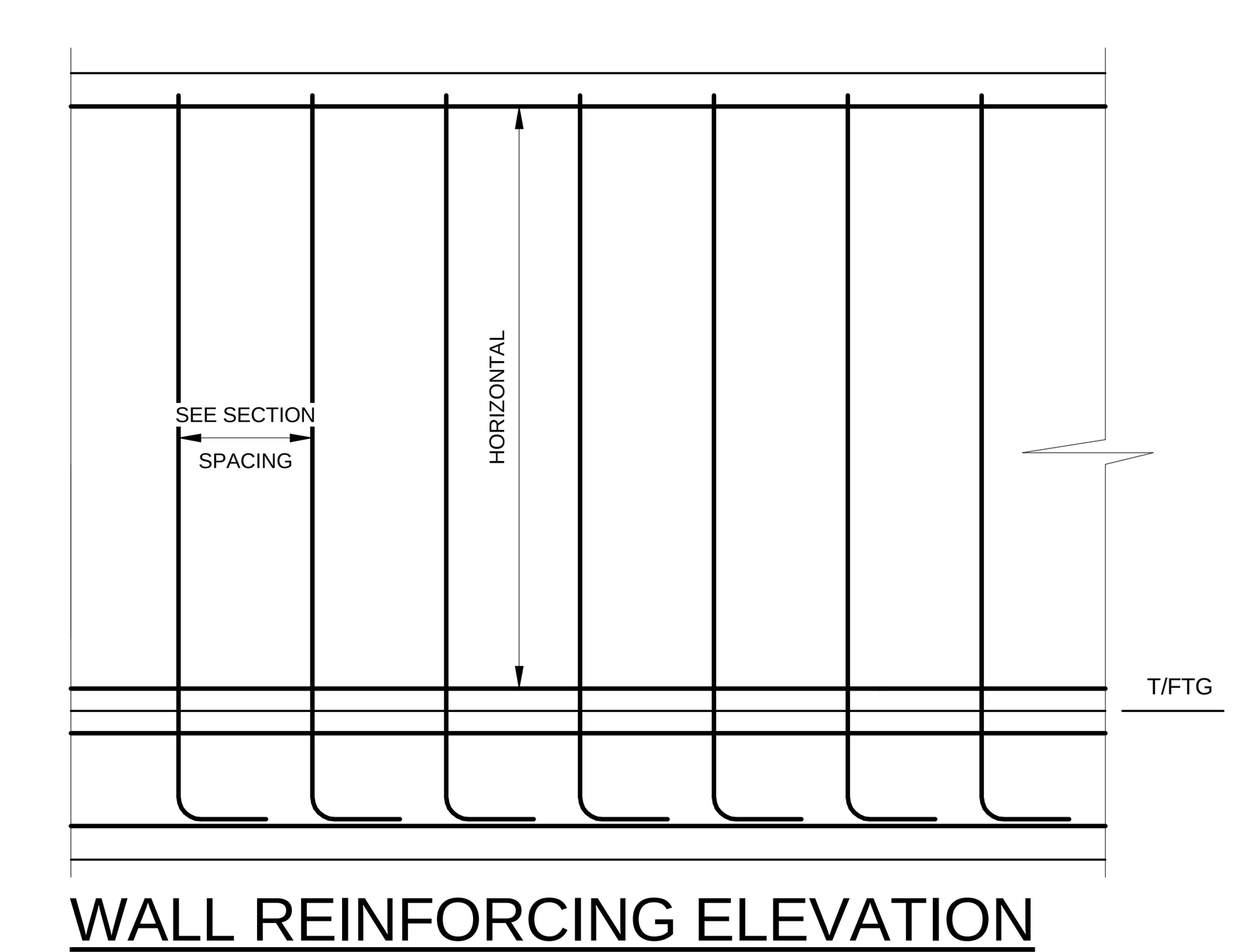
1. All concrete to have 3000 psi compressive strength at 28 days unless otherwise noted.

2. Design parameters to be verified by geotechnical engineer.
   Equivalent fluid pressure : 40.0 pcf (Cantilever)
   Equivalent fluid pressure : 60.0 pcf (Basement)
   Friction factor : 0.4

3. Reinforcing steel:
   A. Shall be detailed, fabricated and placed according to the latest standards of the A.C.I.
   B. Provide corner bars at all corners of the same size and number as the larger of the adjacent bars.
   C. Shall conform to the latest standards of ASTM A 615 Grade 60.
   D. Contractor to submit shop drawings to the Architect for approval prior to fabrication.

4. Wall joints:
   A. Full height vertical weakened plane contraction joints shall be located at 25' o.c. maximum spacing. Alternate longitudinal bars shall be cut exactly opposite such contraction joints.
   B. Keyed full height vertical expansion joints shall be substituted at every fourth contraction joint or 100' maximum. Joints to have asphalt impregnated fiber board filler or similar, with all longitudinal bars cut opposite such expansion joints.
   C. Construction joints between successive pours of concrete to be keyed (metal Keykold or similar).
   D. Construction joint may be substituted for any control joint.

5. Backfill:
   A. DO NOT backfill above retaining wall heel (behind wall) until 7 days after concrete floor slab and concrete paving has been installed
   B. Compact backfill per requirements of the Soils Engineer.



WALL REINFORCING ELEVATION

ISSUED FOR CONSTRUCTION

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GEORGIA REGISTERED PROFESSIONAL ENGINEER No. 32772 BRENDAN F. CROWLEY

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RETAINING WALL SECTIONS

Date:
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SHEET NUMBER
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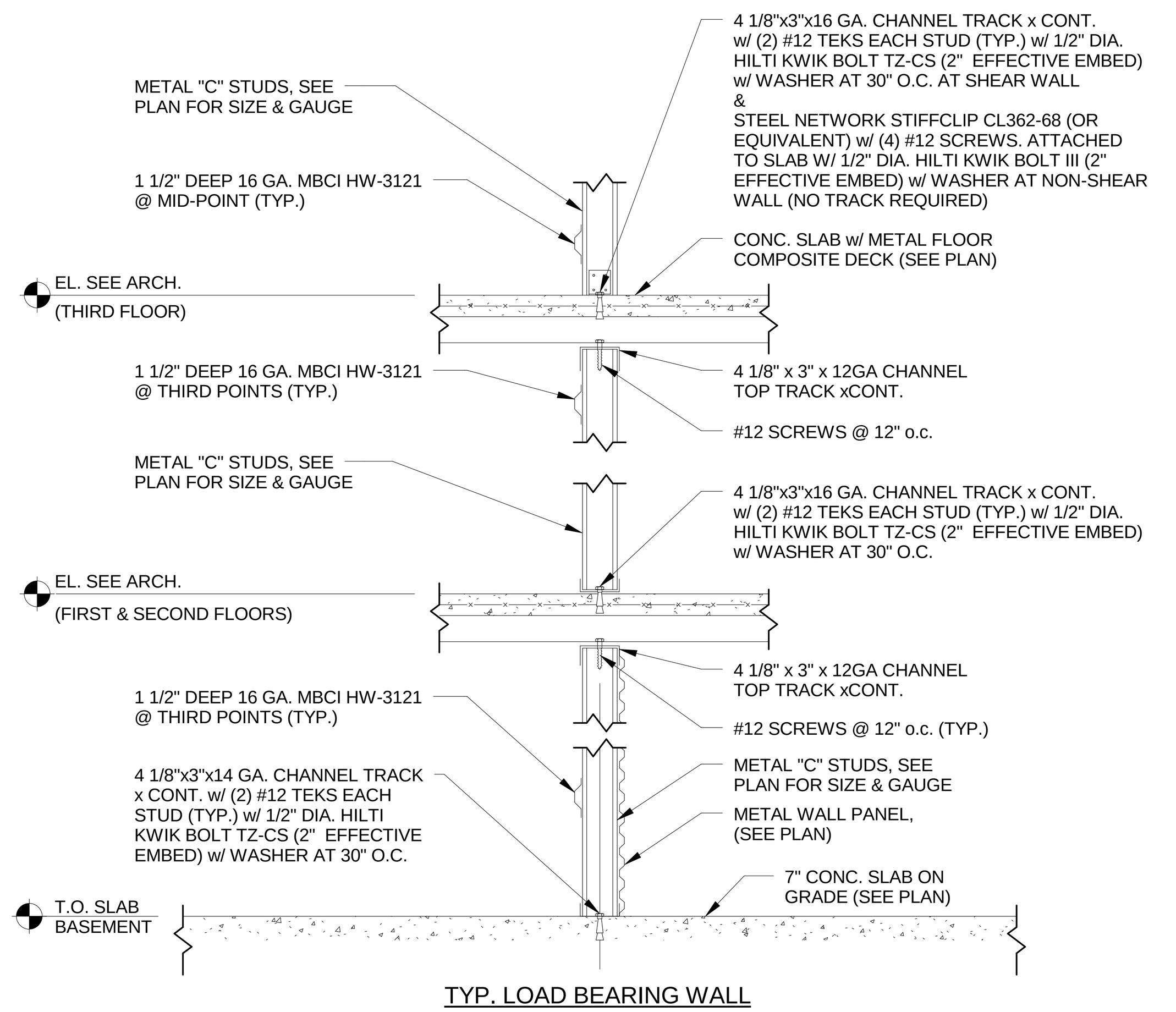
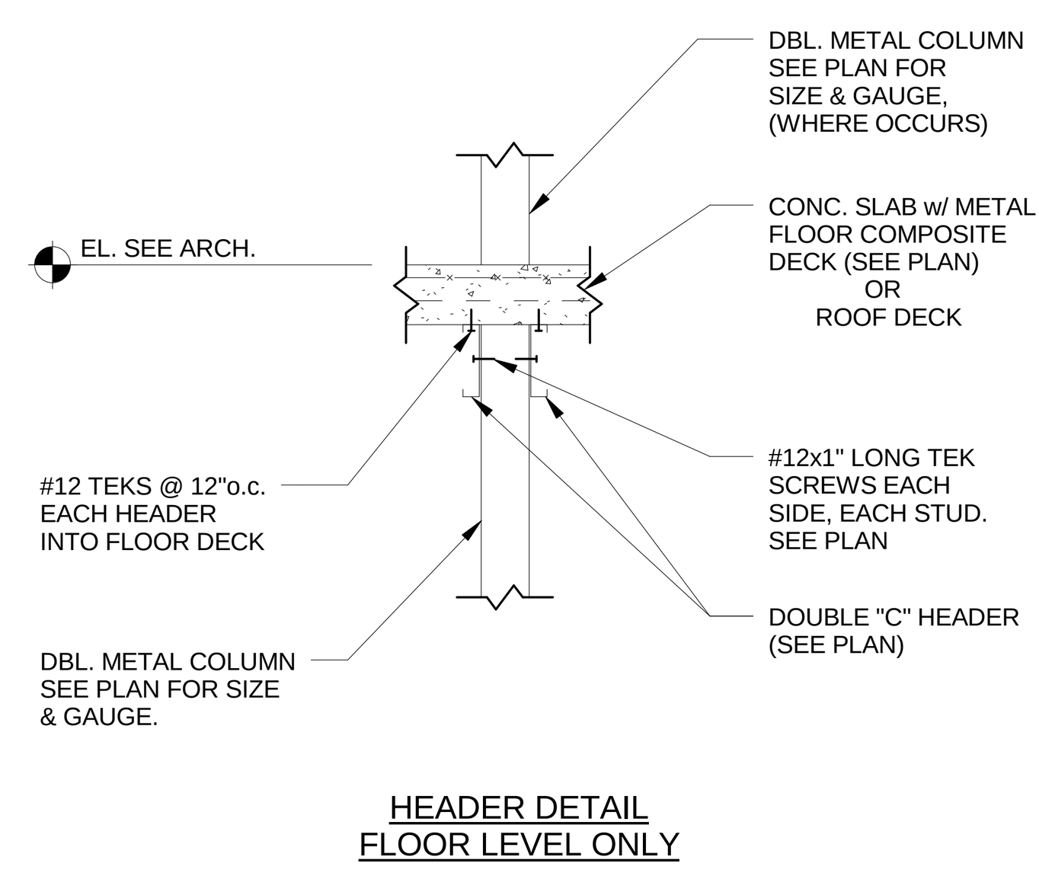
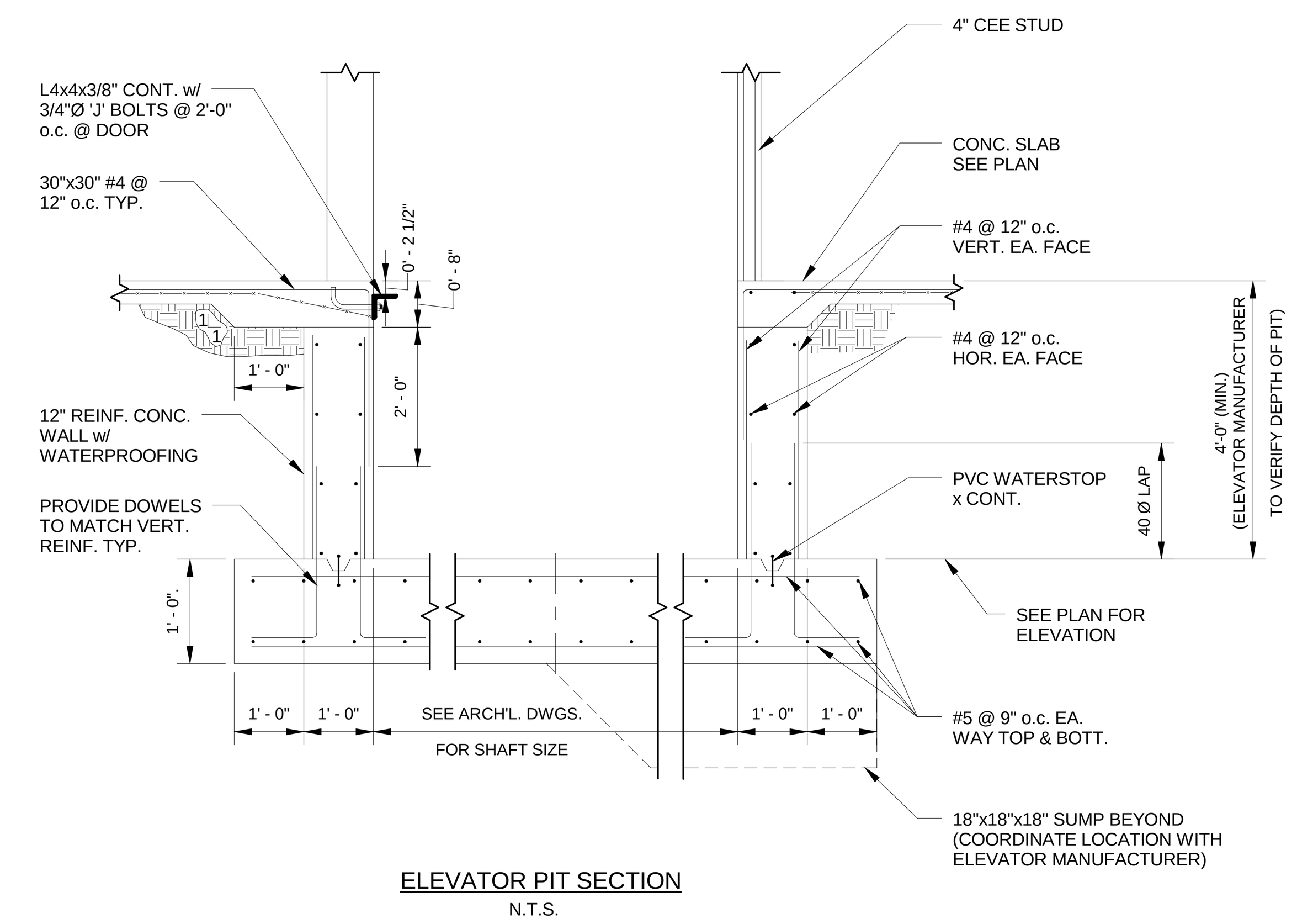


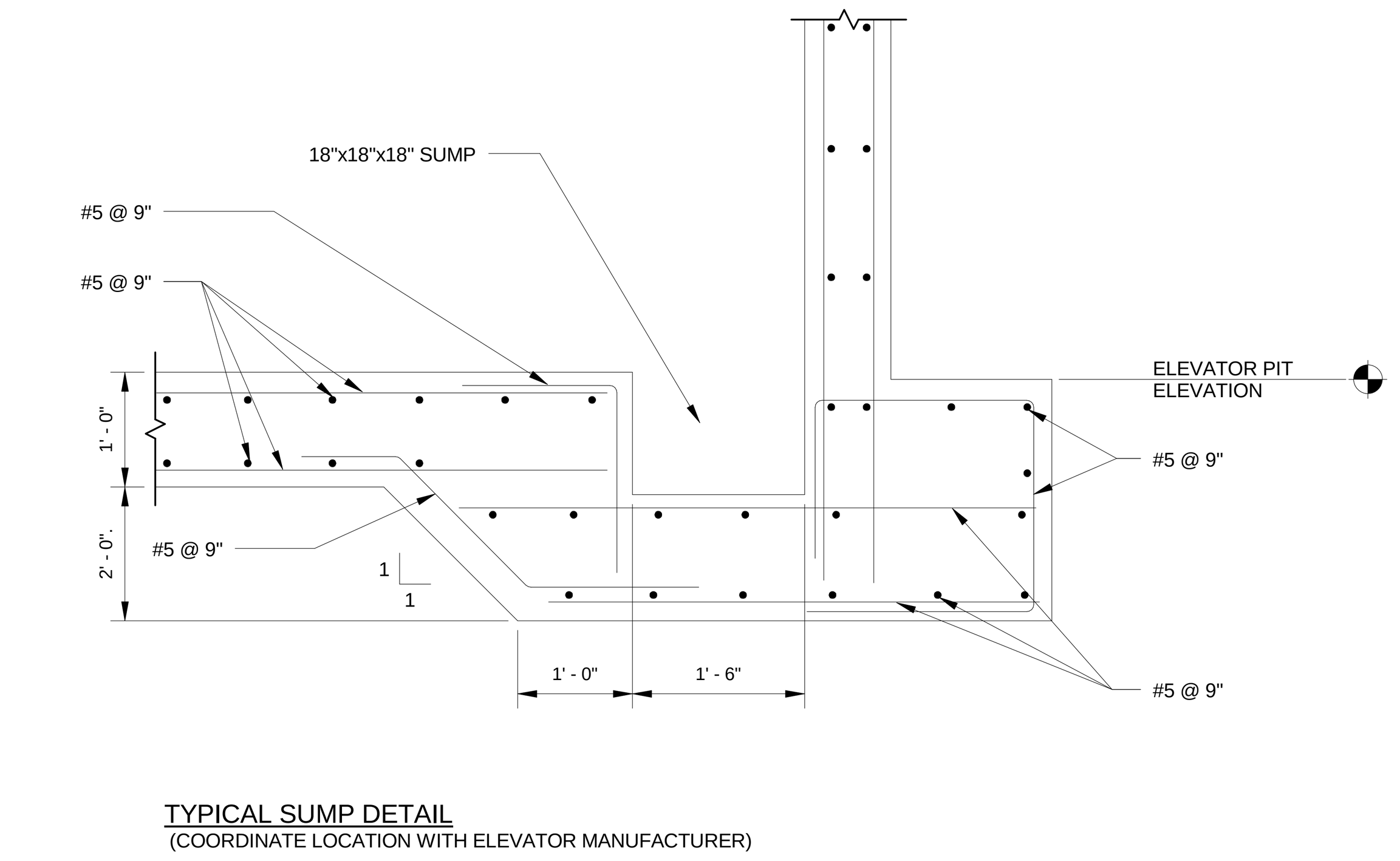
3 S4.1 SECTION
3/4" = 1'-0"



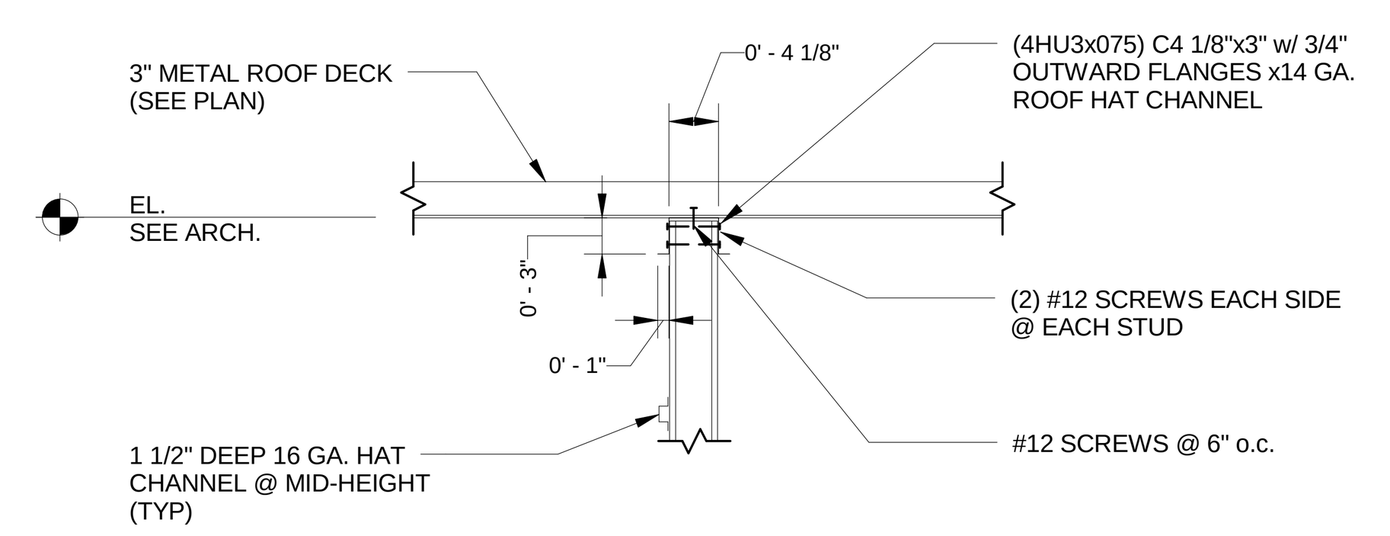
4 HEADER DETAIL
3/4" = 1'-0"



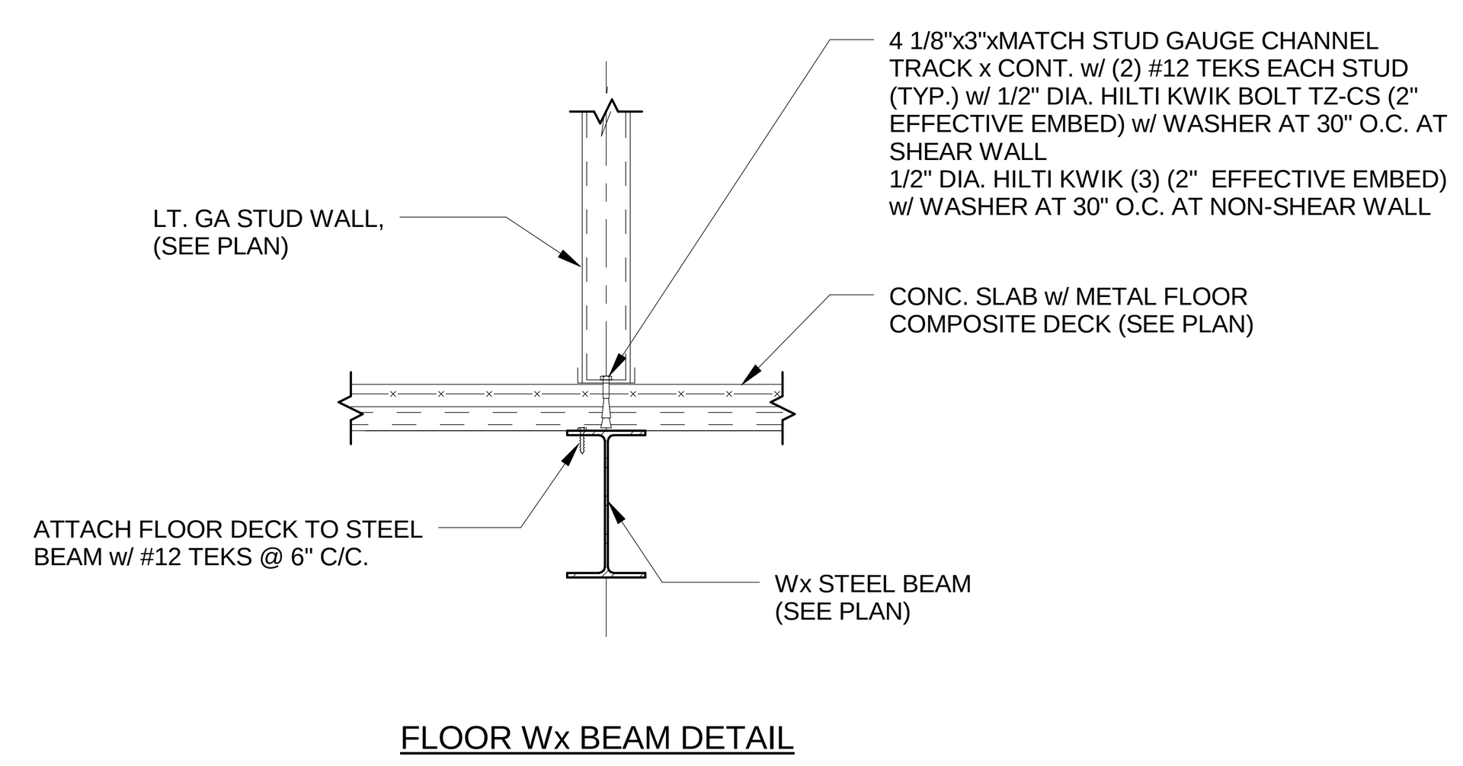
11 S4.1 ELEVATOR PIT
1/2" = 1'-0"



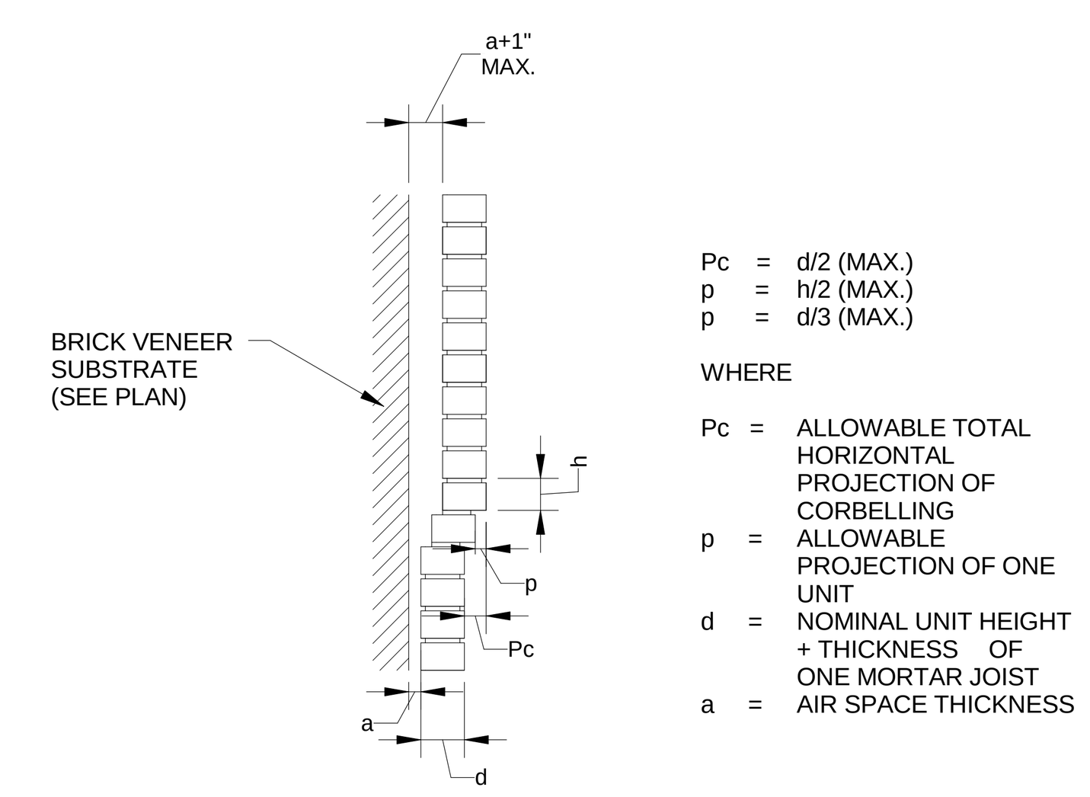
9 S4.1 SUMP DETAIL
3/4" = 1'-0"



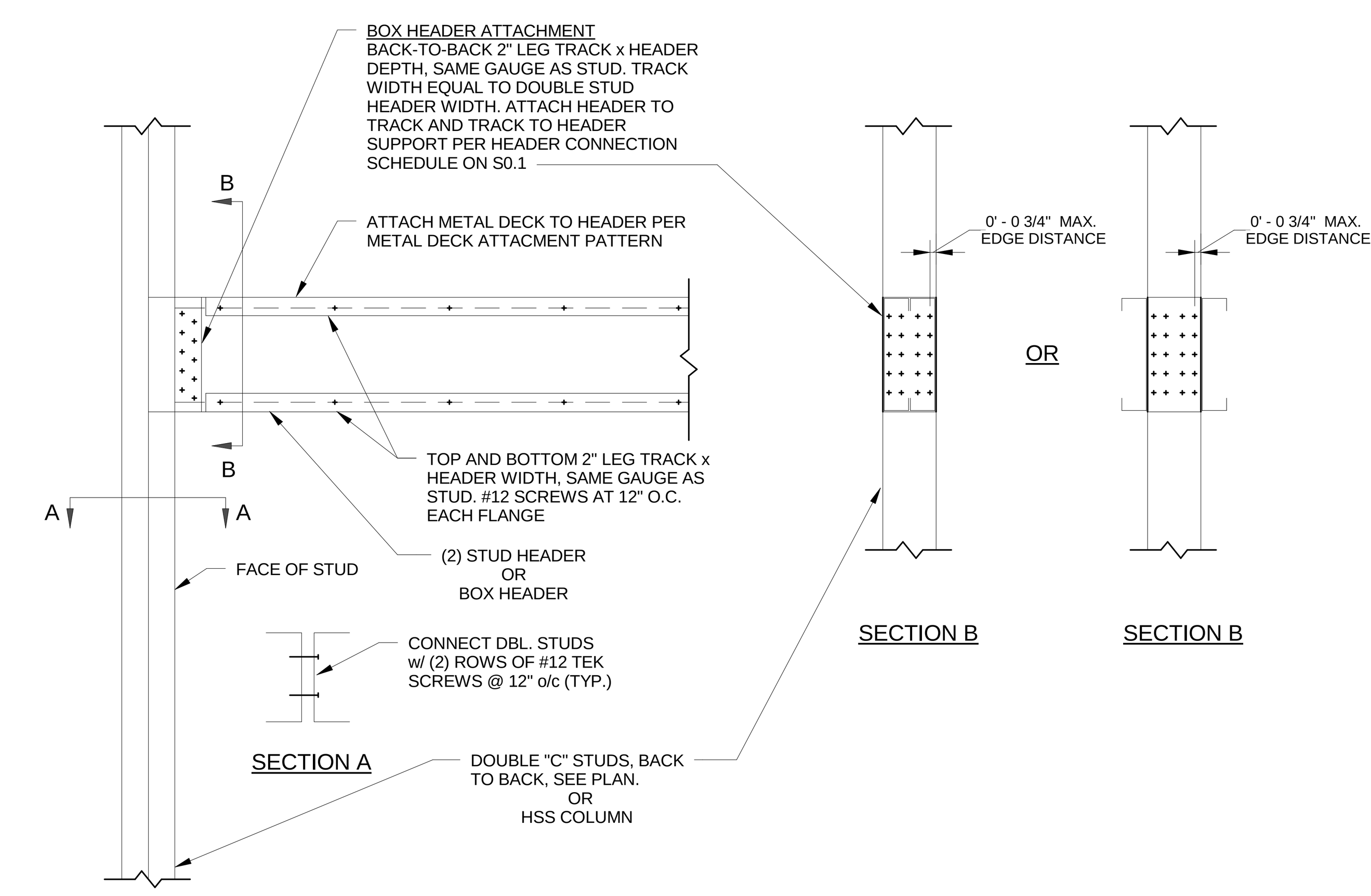
5 S4.1 DECK BEARING
3/4" = 1'-0"



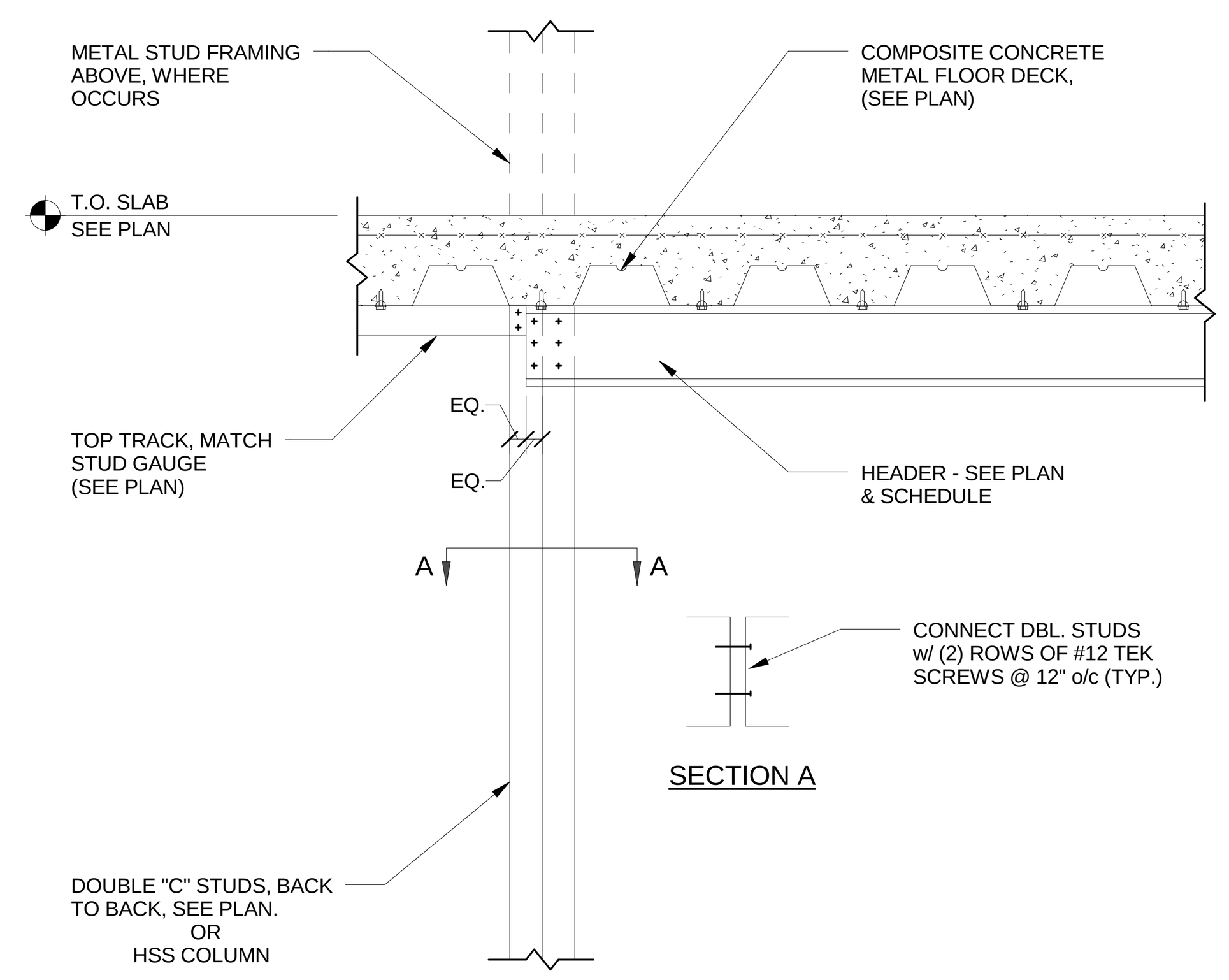
8 DECK BEARING
3/4" = 1'-0"



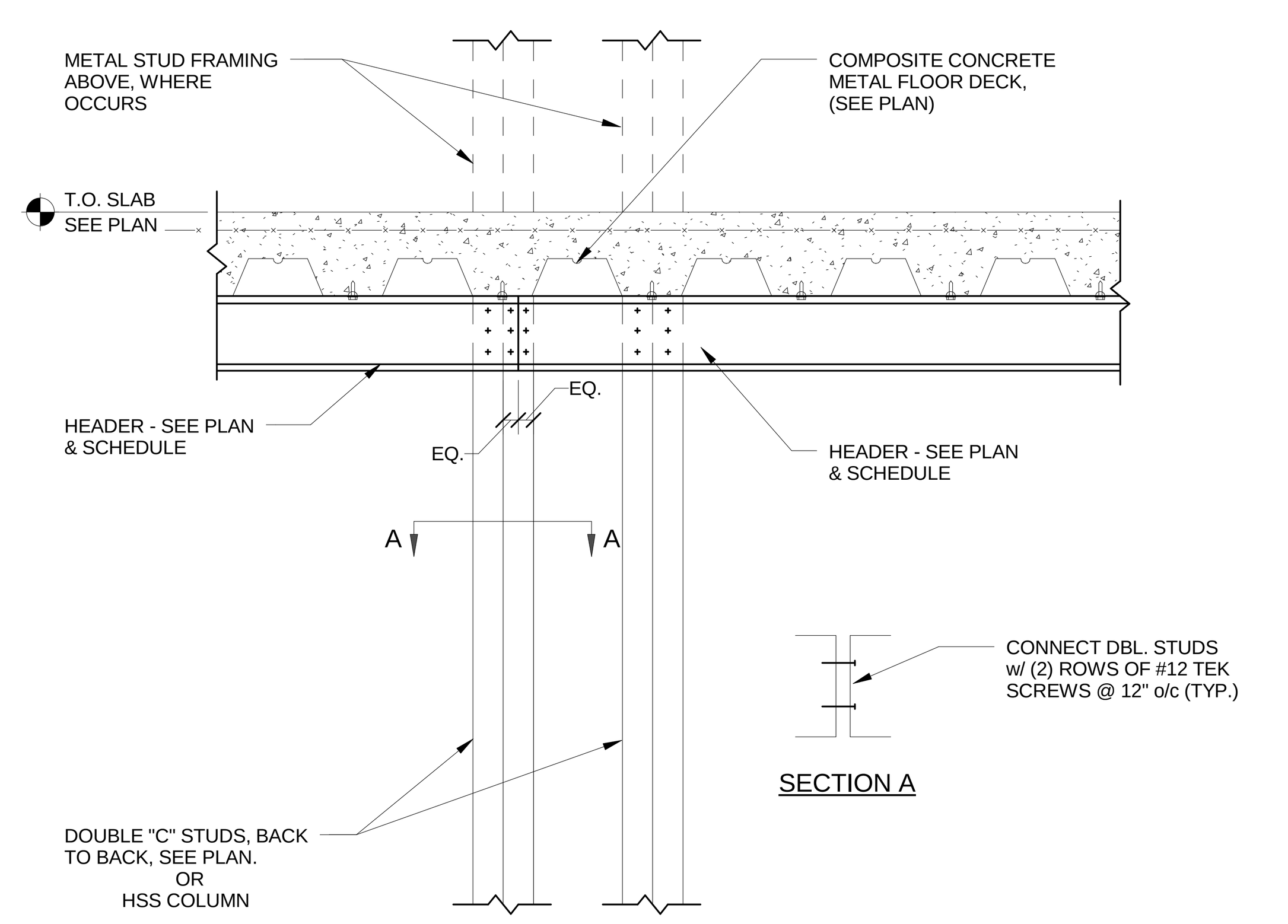
13 S4.1 LIMITS OF BRICK VENEER CORBELLING
3/4" = 1'-0"



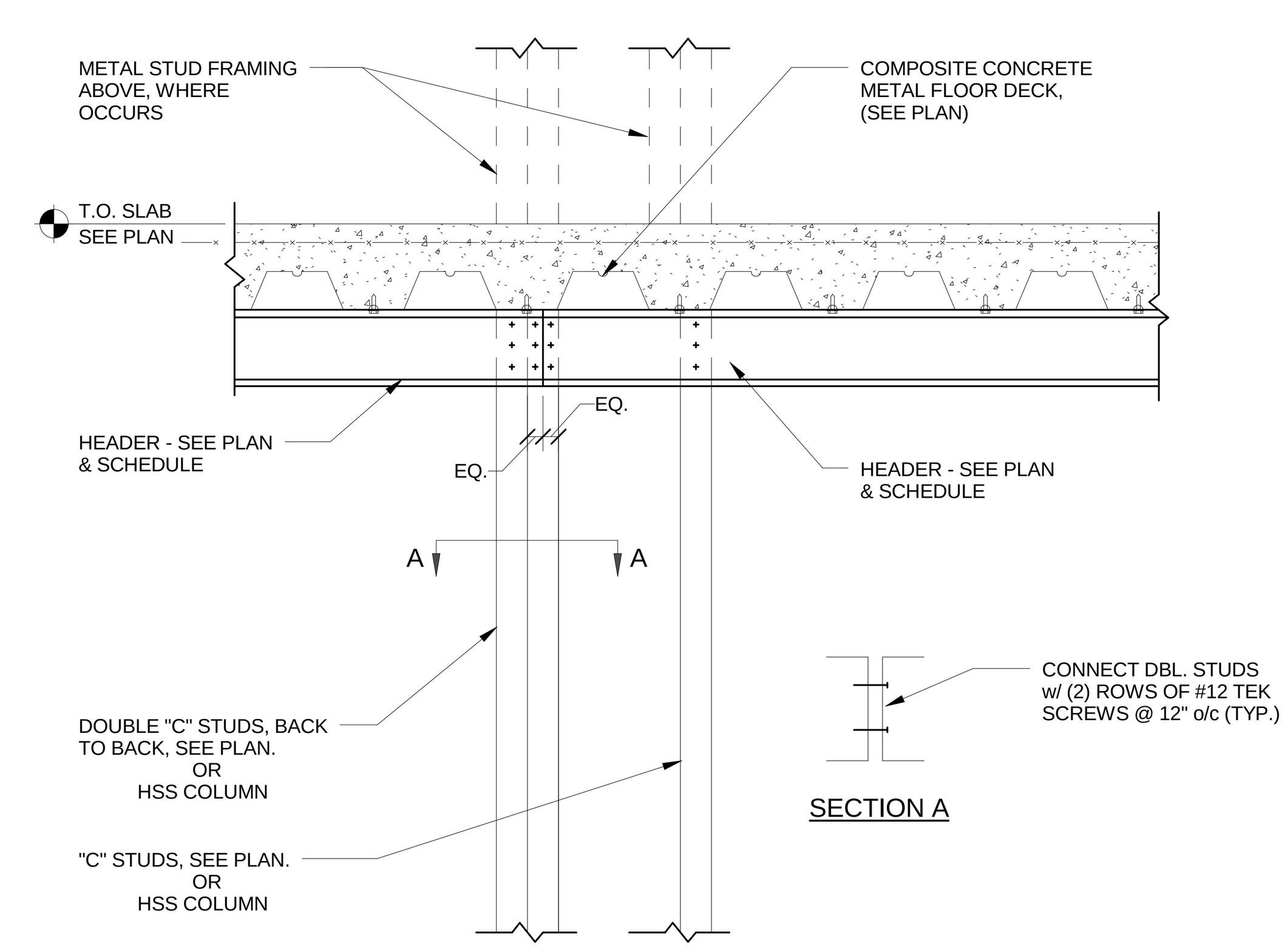
12 S4.1 BOX HEADER BEARING DETAIL
3/4" = 1'-0"



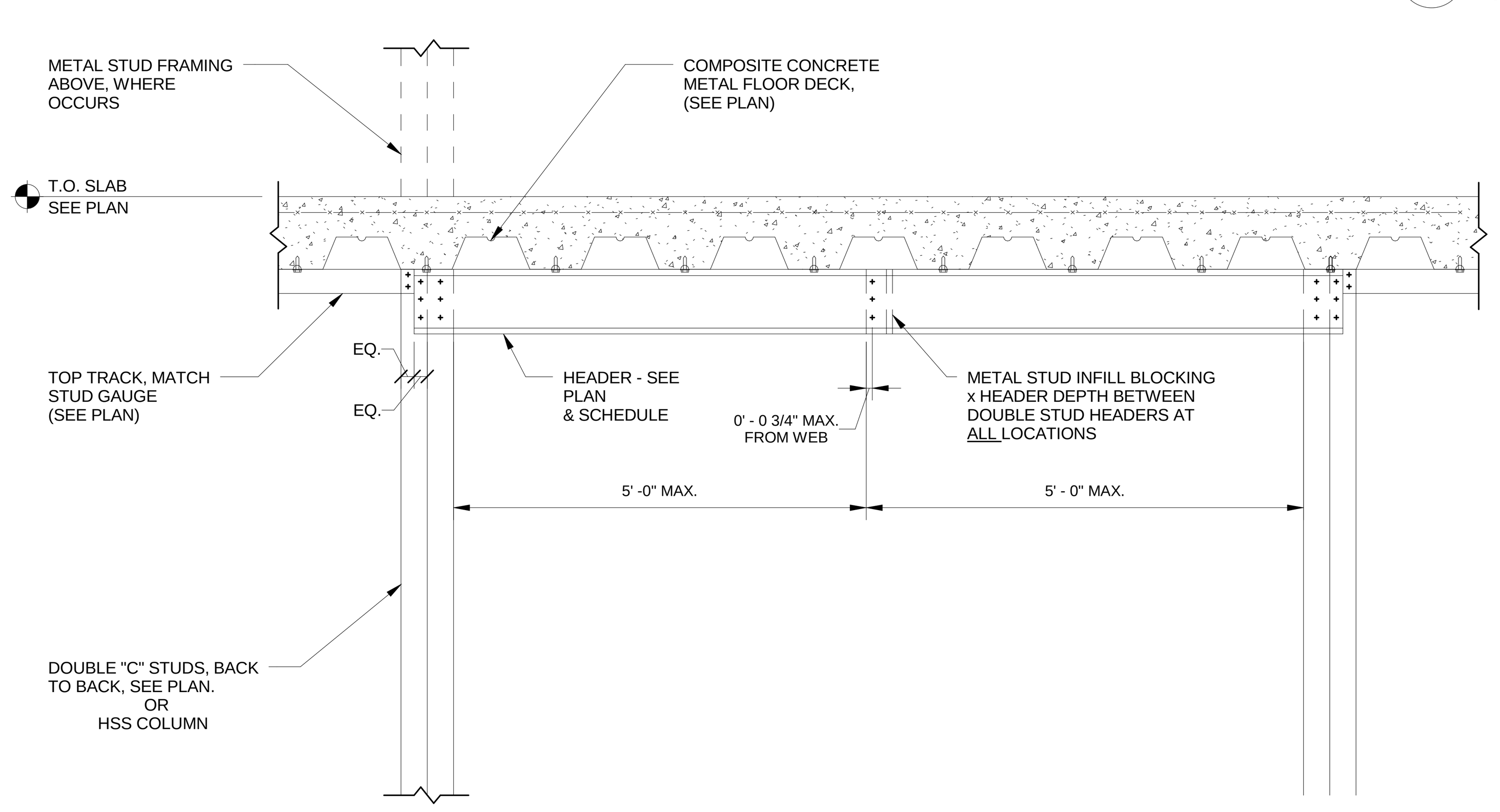
6 S4.1 HEADER BEARING DETAIL
3/4" = 1'-0"



7 S4.1 CONTINUOUS HEADER BEARING DETAIL
3/4" = 1'-0"



10 S4.1 CONTINUOUS HEADER BEARING DETAIL
3/4" = 1'-0"



1 S4.1 STUD HEADER BLOCKING DETAIL
3/4" = 1'-0"

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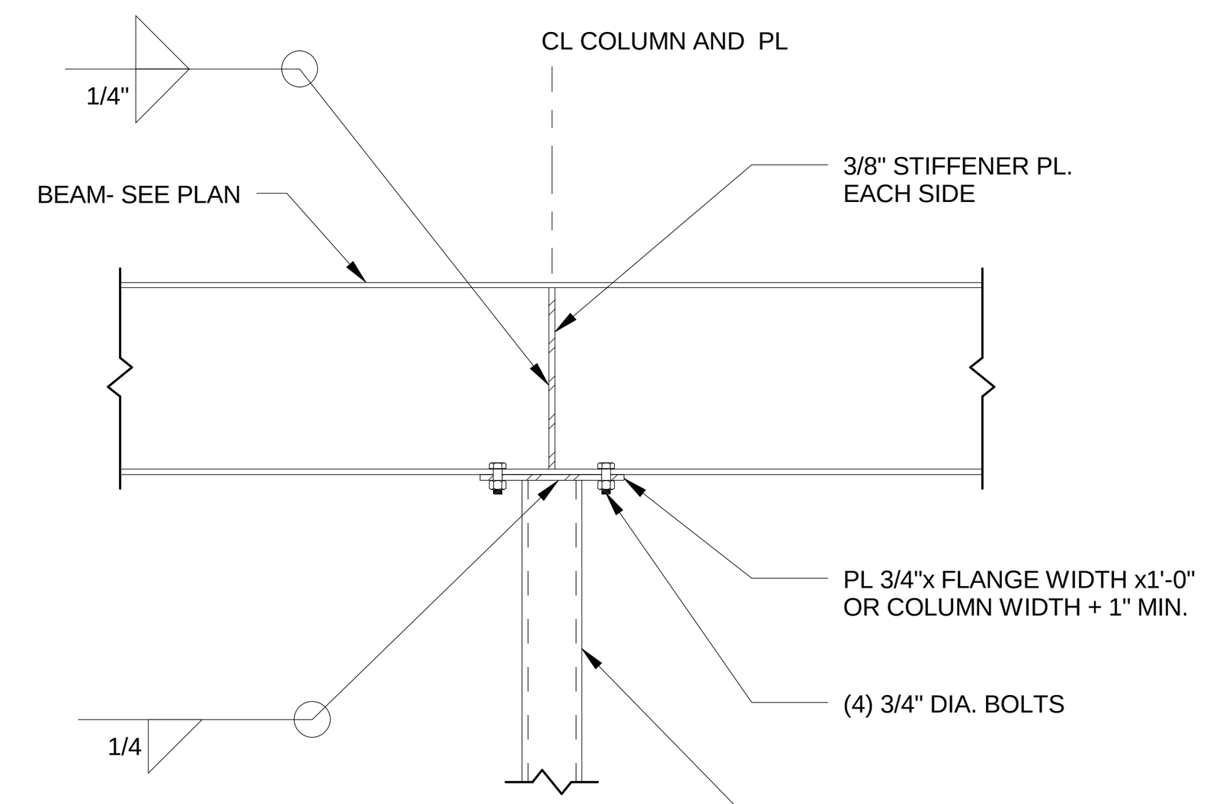
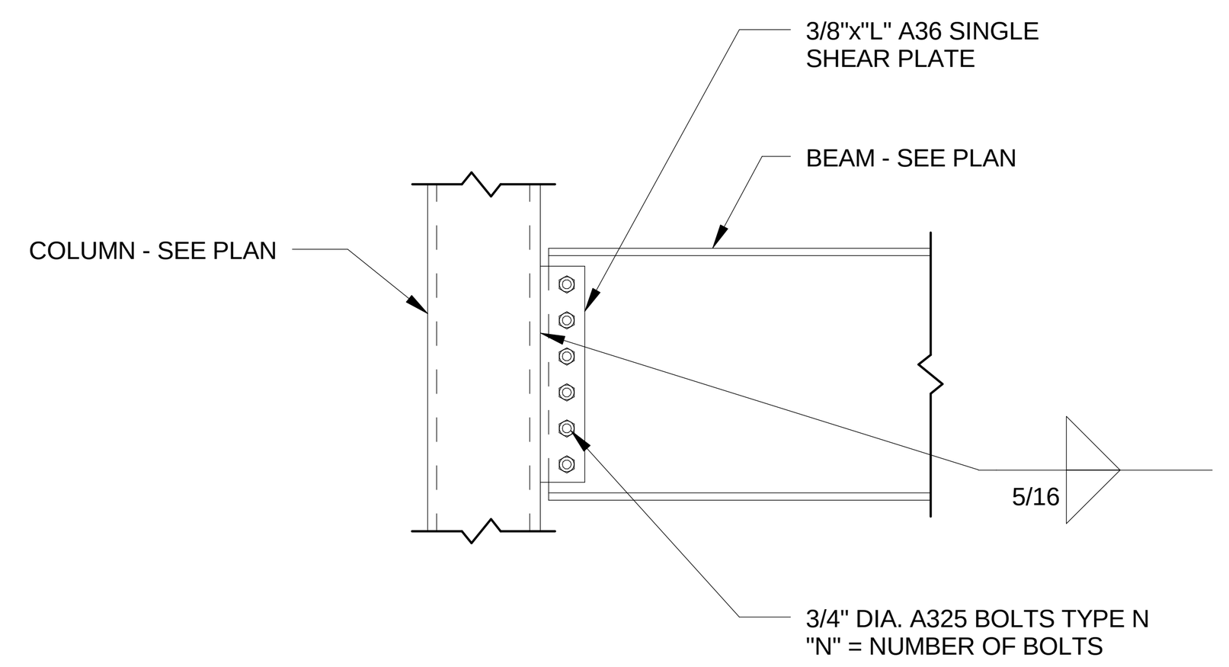
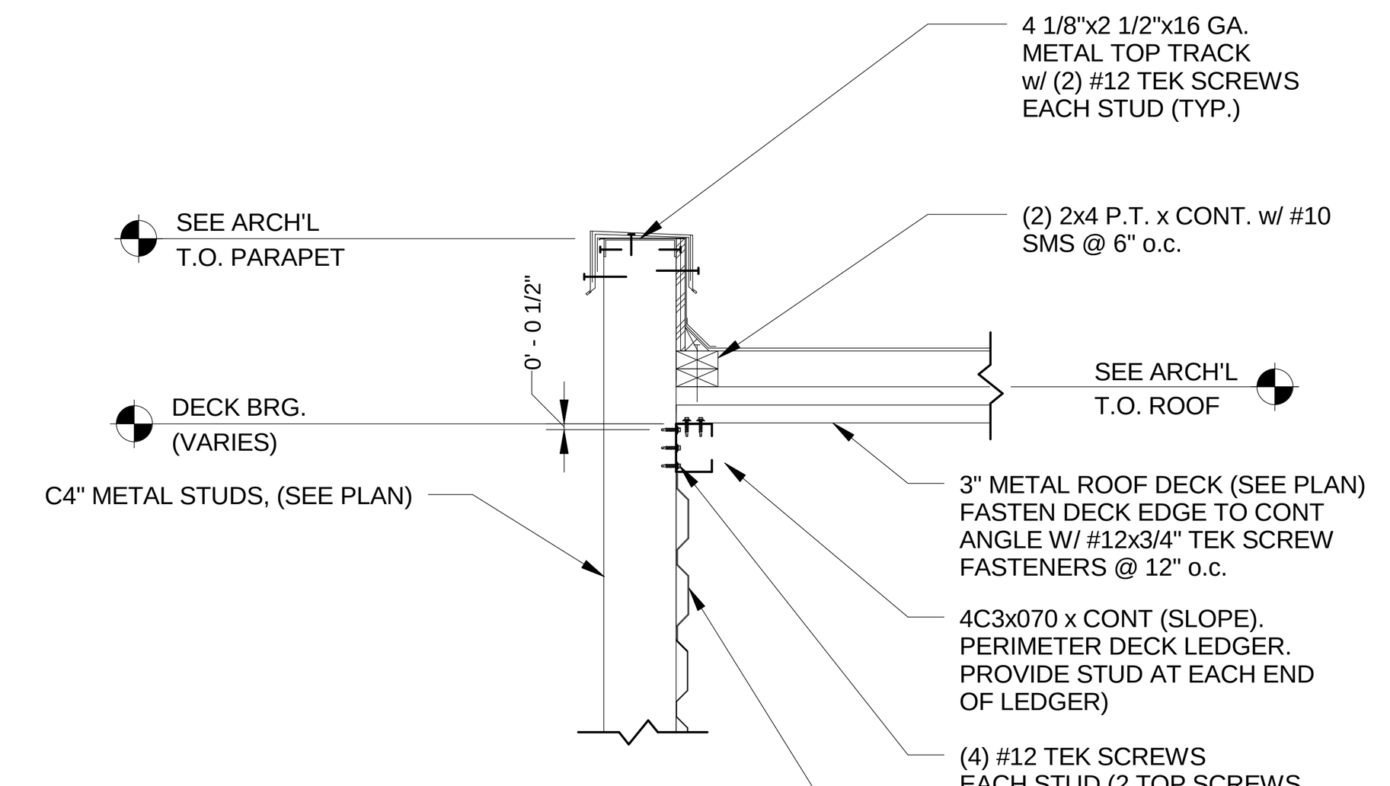


1 S4.2 BEAM OVER COLUMN DETAIL
3/4" = 1'-0"



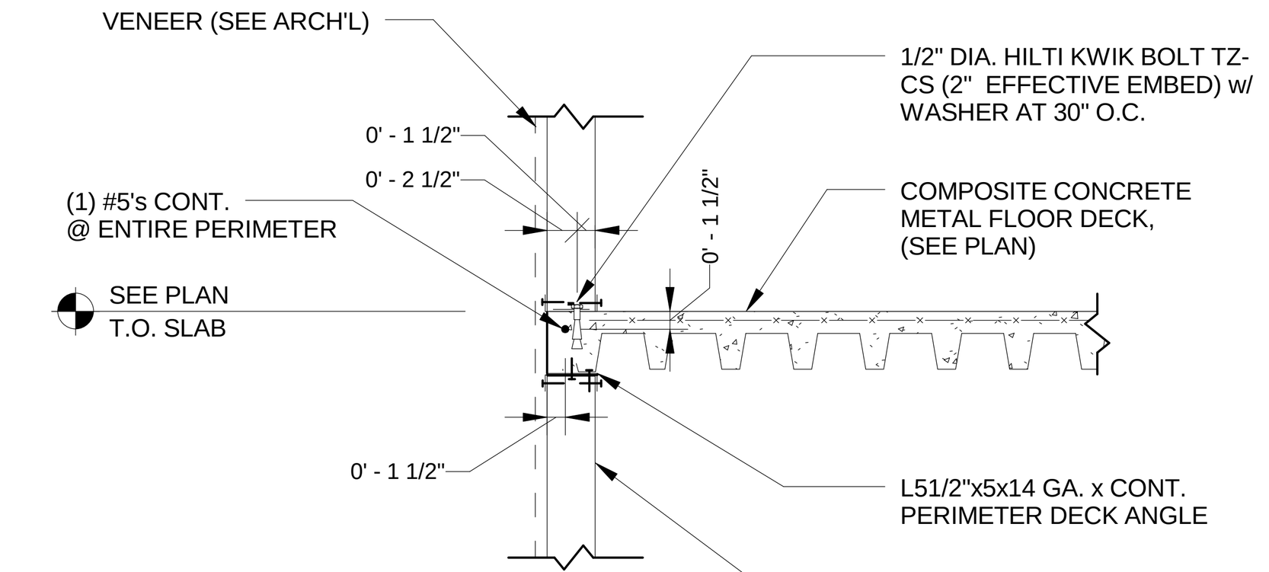
BEAM SIZE	PLATE LENGTH "L"	NUMBER OF BOLTS "N"
W24	1'-9"	7
W21	1'-6"	6
W18	1'-3"	5
W16	1'-0"	4
W12-W14	0'-9"	3
W8-W10	0'-6"	2

2 S4.2 BEAM TO COLUMN DETAIL
3/4" = 1'-0"

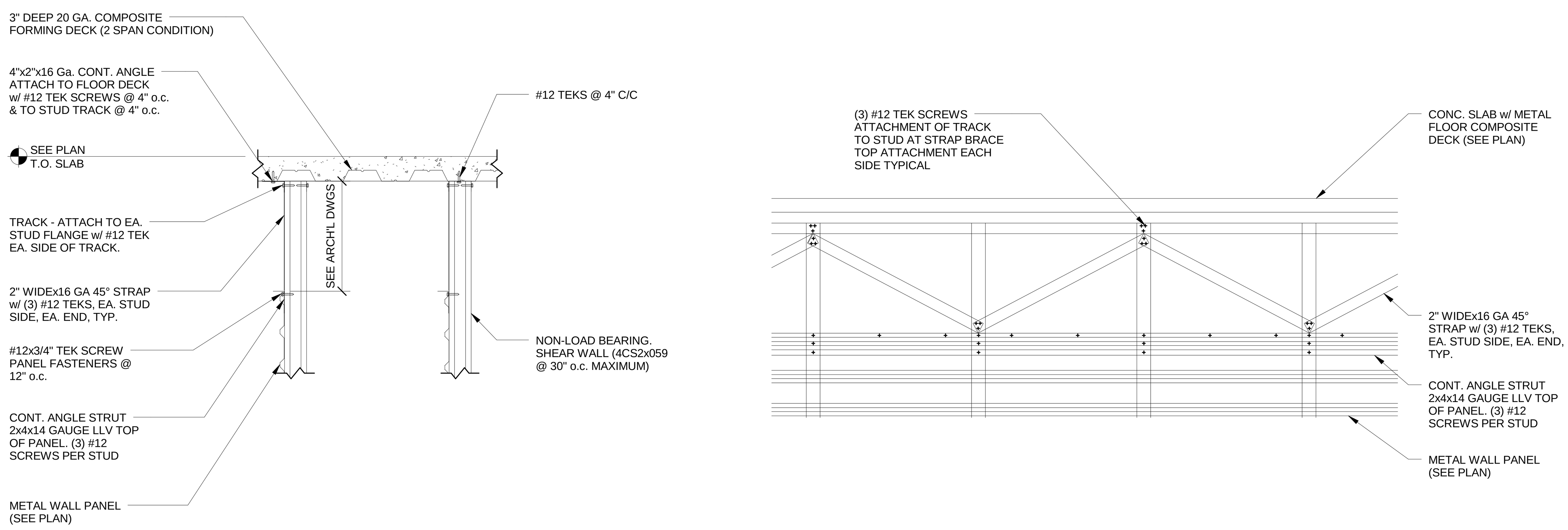


NOTE:
DECK SPAN DIRECTION CHANGES AT SIMILAR CONDITION.

3 S4.2 ROOF DECK BEARING
3/4" = 1'-0"



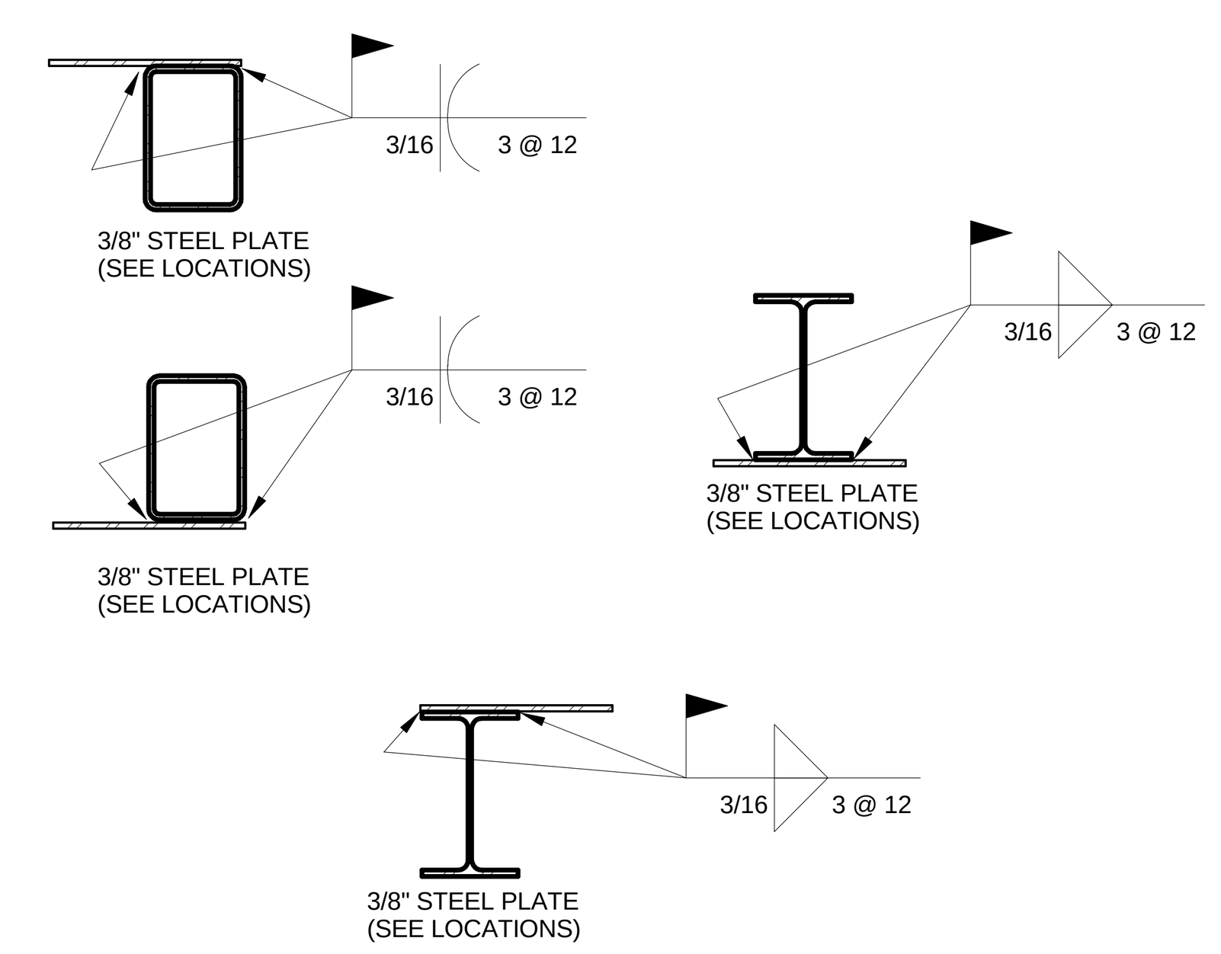
4 S4.2 FLOOR DECK BEARING
3/4" = 1'-0"



NOTE:
16 GA. STRAP NOT NEEDED IF WALL PANEL IS CONTINUOUS TO UNDERSIDE OF DECK, FULLY BRACING THE STUD.

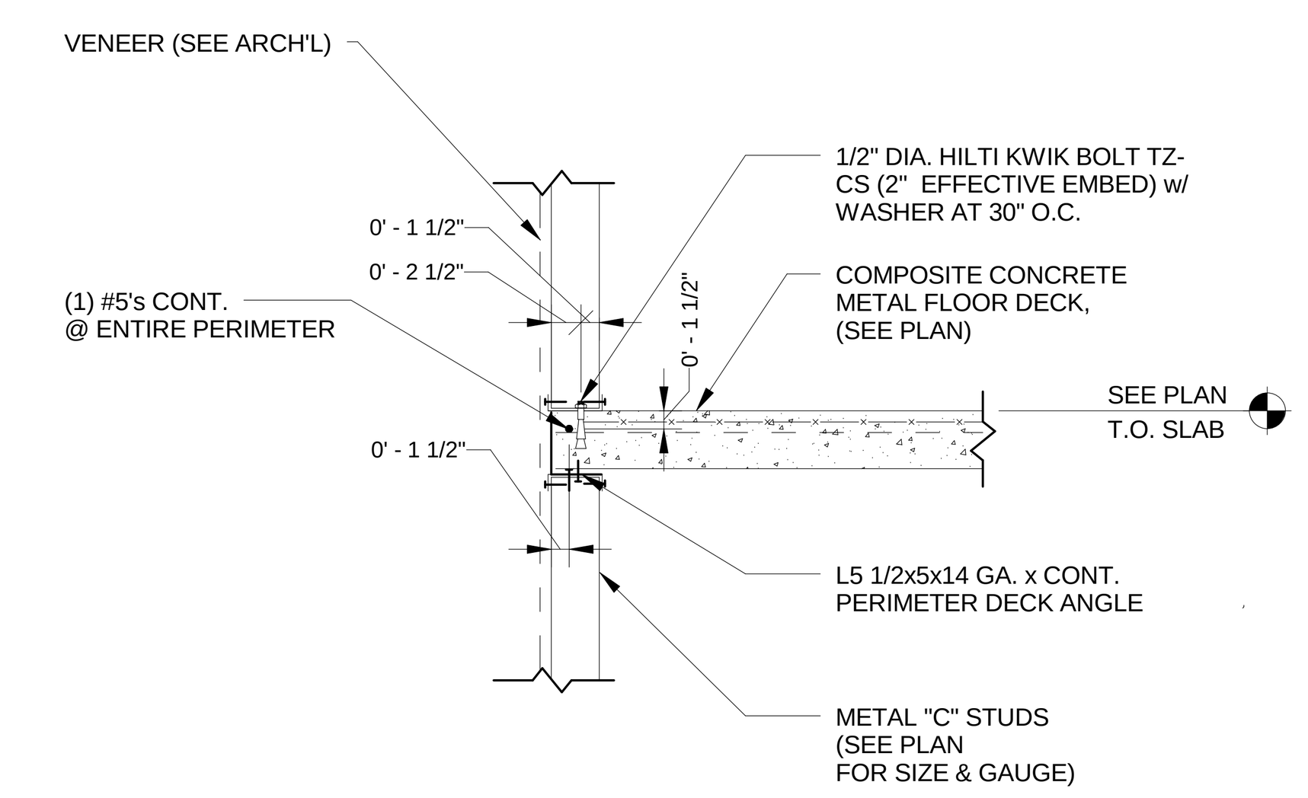
NOTE:
FASTEN PANELS TO BOTTOM TRACK W/ #12x3/4" TEK SCREW PANEL FASTENERS @ 12" o.c.

7 S4.2 SHEAR WALL STARP BRACING ABOVE METAL PANEL (TRANSVERSE SHEAR WALL)
3/4" = 1'-0"

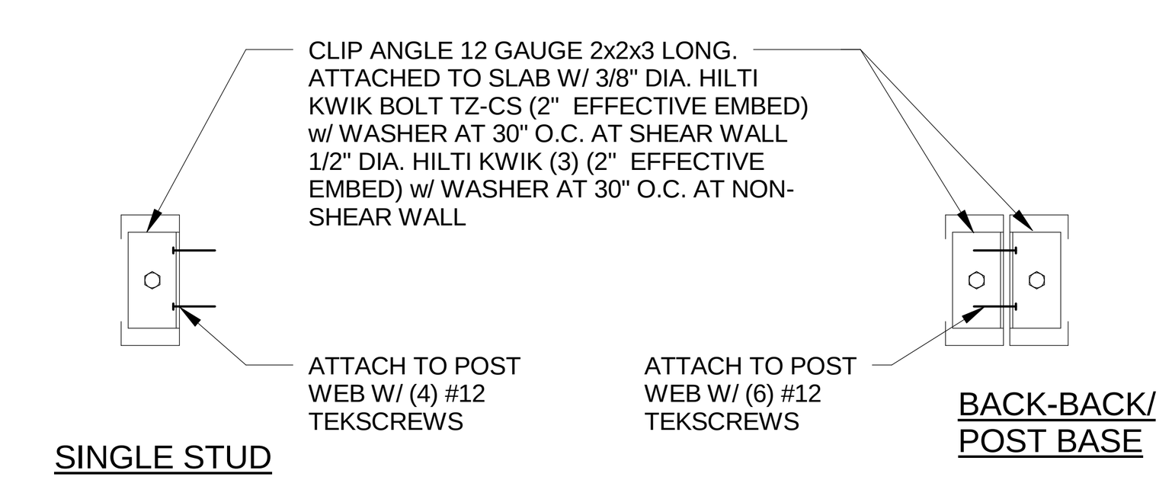


SEE APPLICABLE ARCH. SECTION FOR PLATE LAYOUT AND LOCATION

11 S4.2 TYPICAL PLATE WELD DETAIL
3/4" = 1'-0"

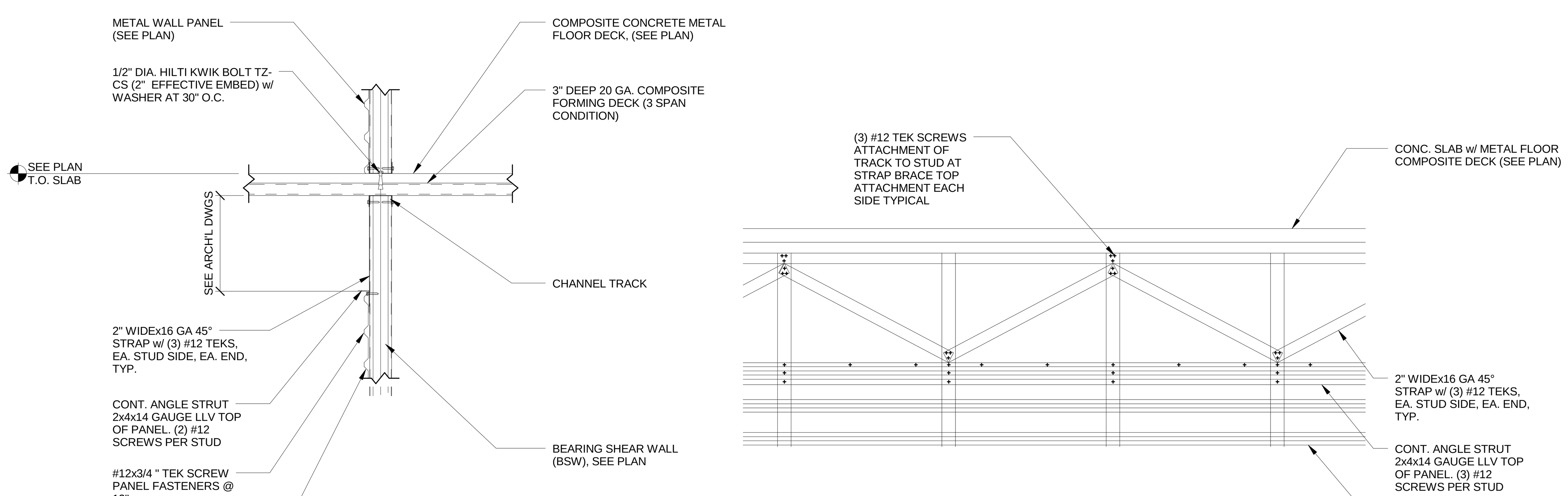


5 S4.2 FLOOR DECK BEARING
3/4" = 1'-0"



NOTE:
TRACK NOT SHOWN FOR CLARITY

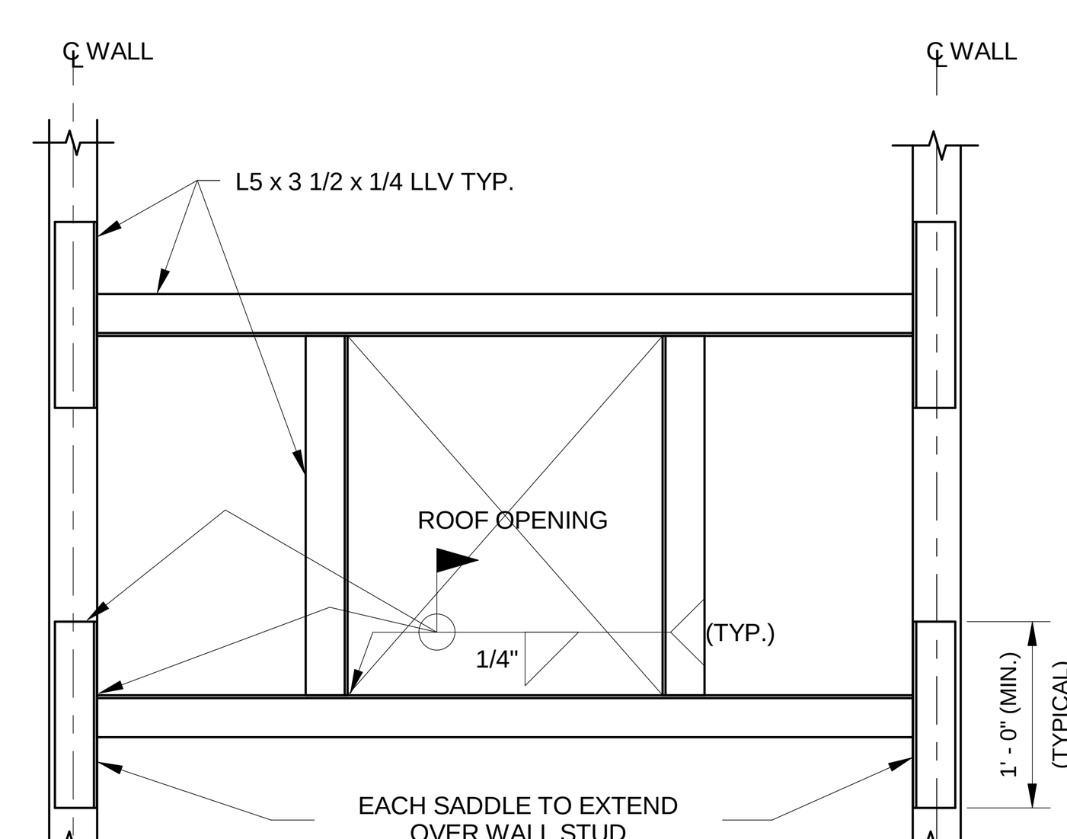
10 S4.2 POST BASE ATTACHMENT TO SLAB
3/4" = 1'-0"



NOTE:
16 GA. STRAP NOT NEEDED IF WALL PANEL IS CONTINUOUS TO UNDERSIDE OF DECK, FULLY BRACING THE STUD.

NOTE:
FASTEN PANELS TO BOTTOM TRACK W/ #12x3/4" TEK SCREW PANEL FASTENERS @ 12" o.c.

8 S4.2 SHEAR WALL STARP BRACING ABOVE METAL PANEL
3/4" = 1'-0"



NOTE:
1. CONTRACTOR SHALL COORDINATE LOCATION & SIZE OF ALL ROOF OPENINGS WITH EQUIPMENT SELECTED

9 S4.2 TYP. ROOF OPENING DETAIL
3/4" = 1'-0"

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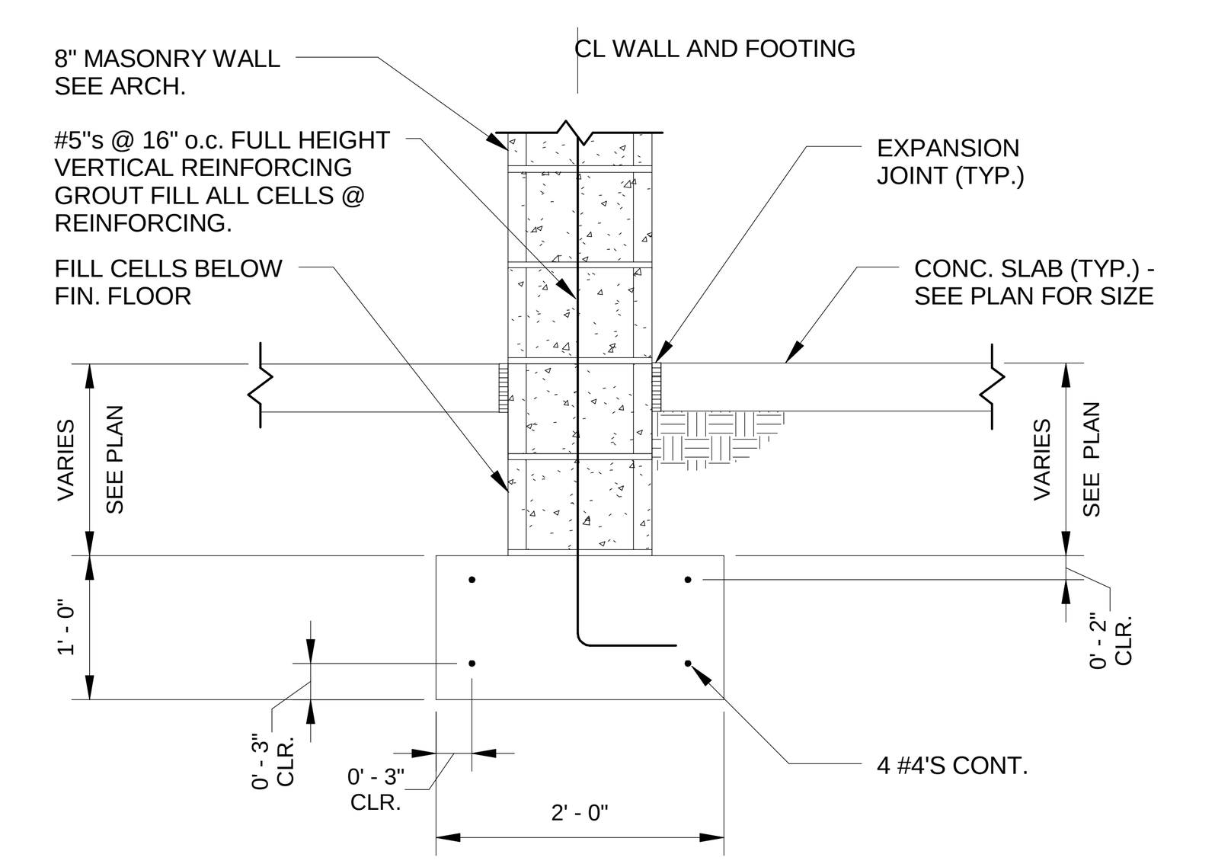
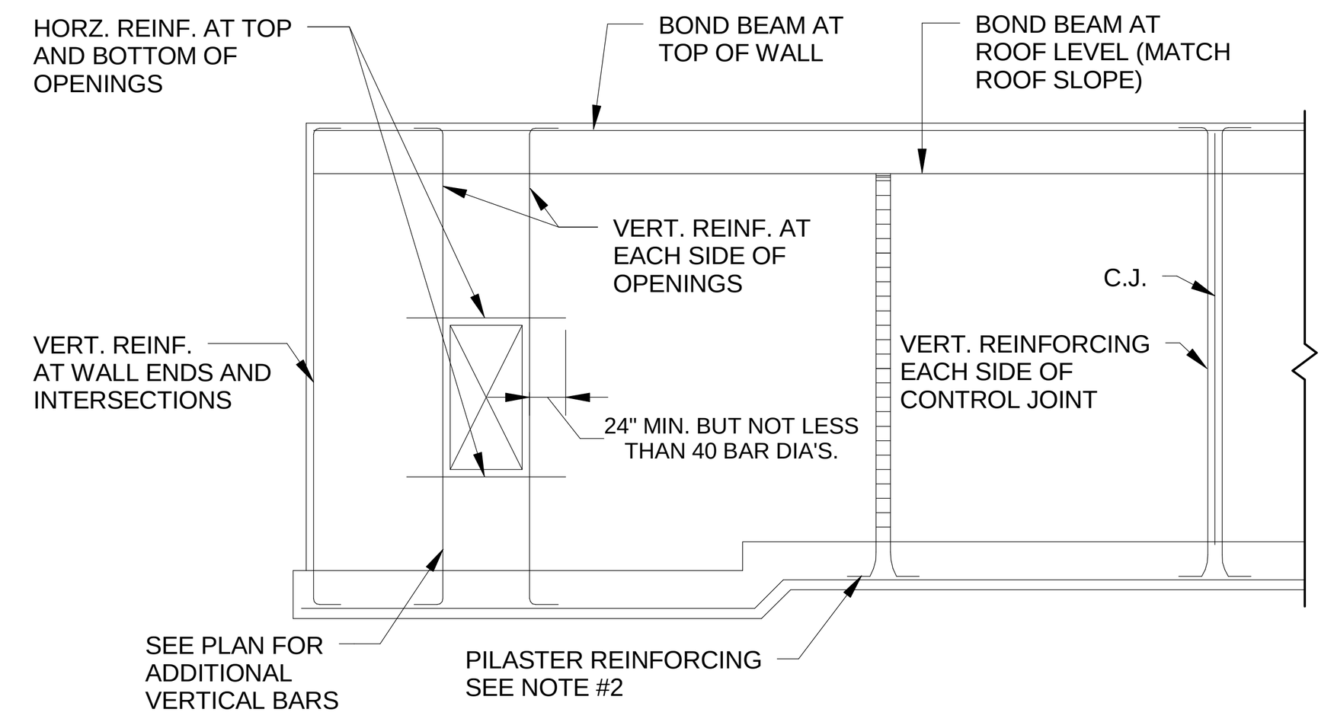


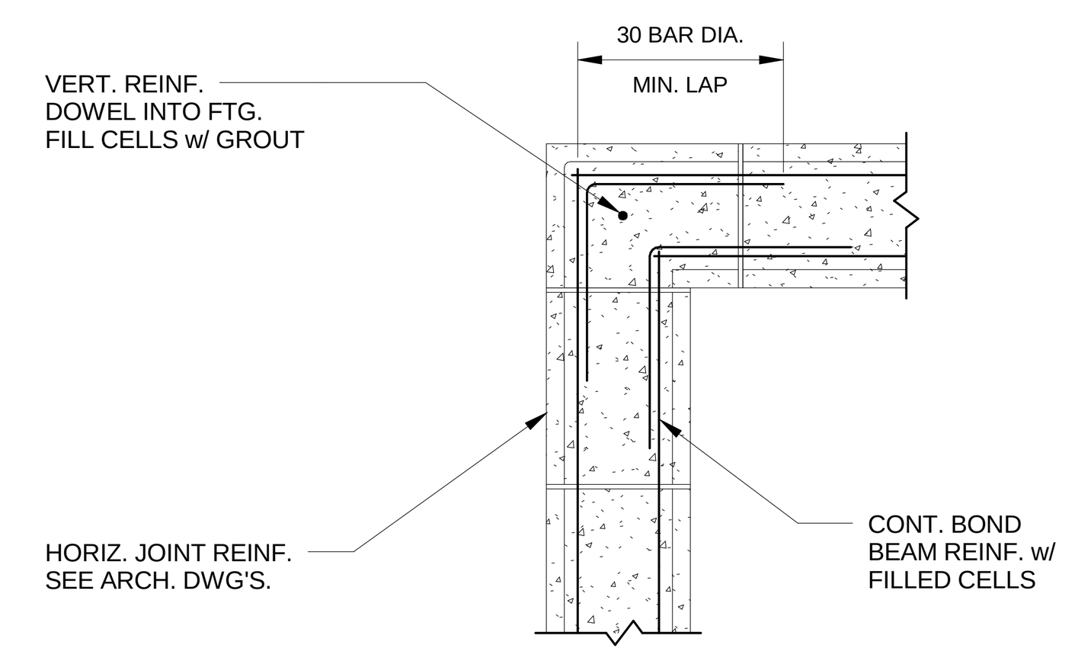
NOTE:
VERTICAL WALL REINFORCING SHALL BE CONTINUOUS THROUGH ALL BOND BEAMS IN WALL. PROVIDE BOND BEAM BLOCK WITH KNOCKOUT IN BOTTOM.

1 S4.3 WALL FOOTING SECTION
3/4" = 1'-0"

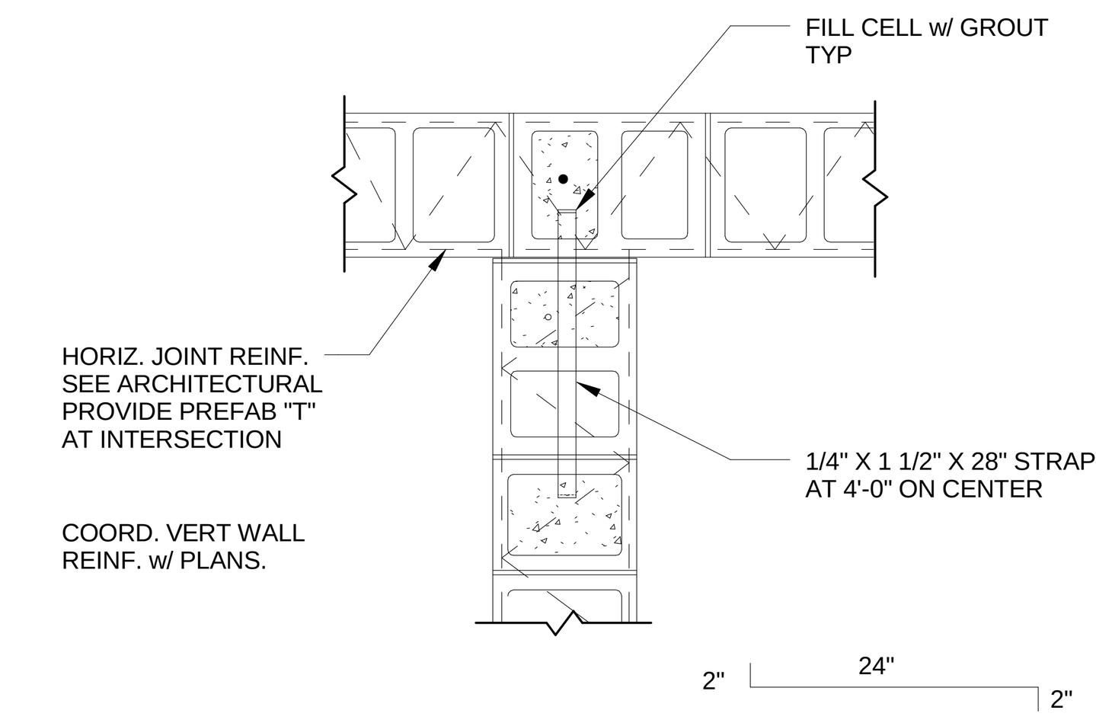


NOTES:
1. REINFORCING SHOWN SHALL BE MINIMUM #4 RE-BAR UNLESS SHOWN OTHERWISE ON PLANS AND DETAILS.
2. SEE PLANS FOR PILASTER LOCATIONS. SEE DETAILS FOR REINFORCING SIZE AND TIE SPACING. DOWEL VERTICAL REINFORCING INTO FOOTING.

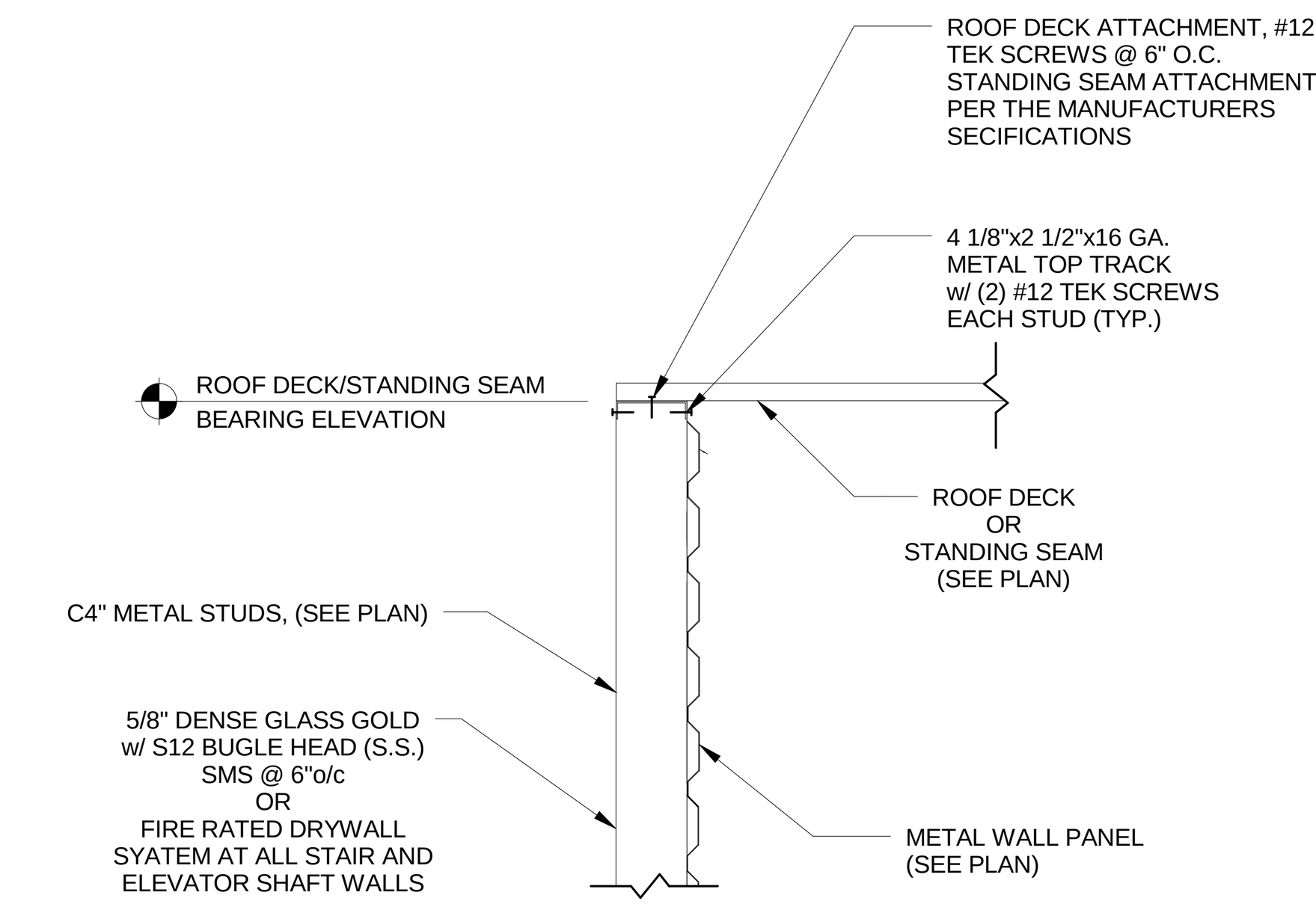
2 S4.3 MASONRY WALL REINFORCING
3/4" = 1'-0"



3 S4.3 CORNER WALL DETAIL
3/4" = 1'-0"

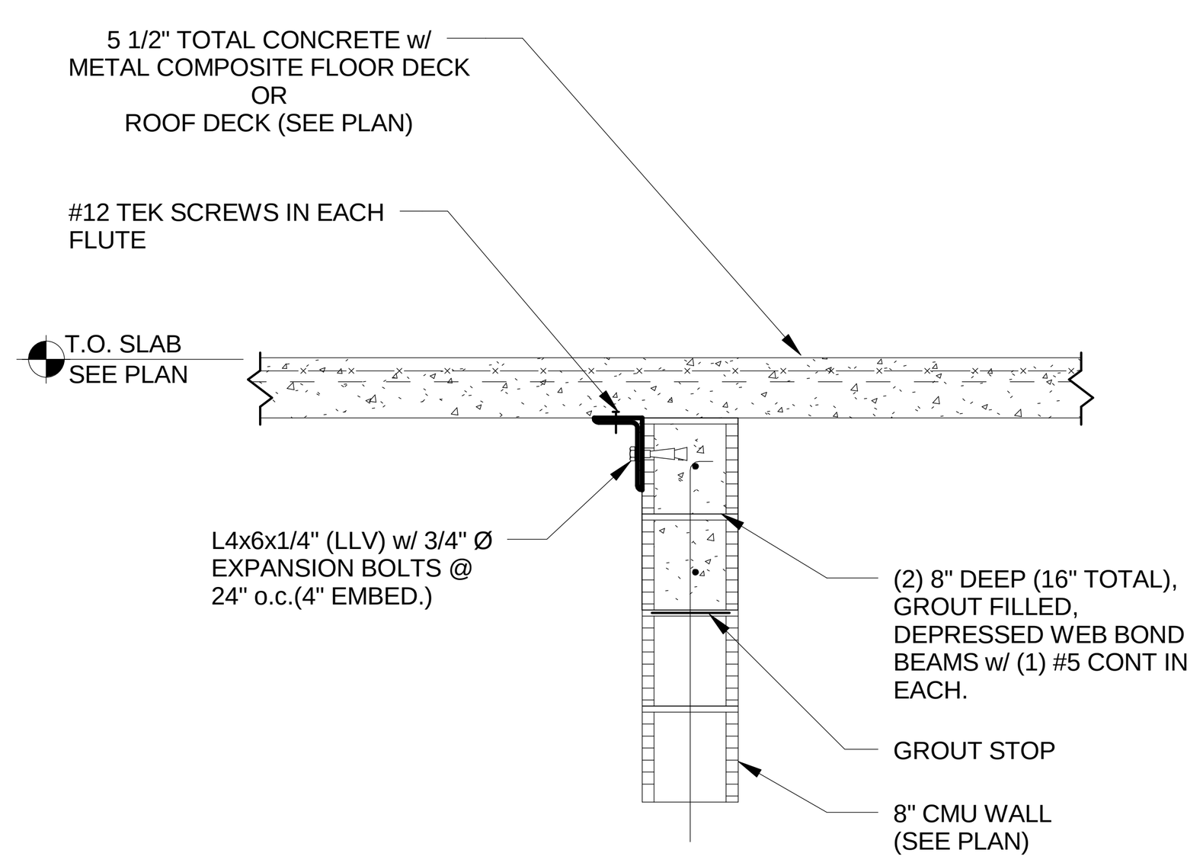


4 S4.3 INTERSECTING WALL DETAIL
3/4" = 1'-0"

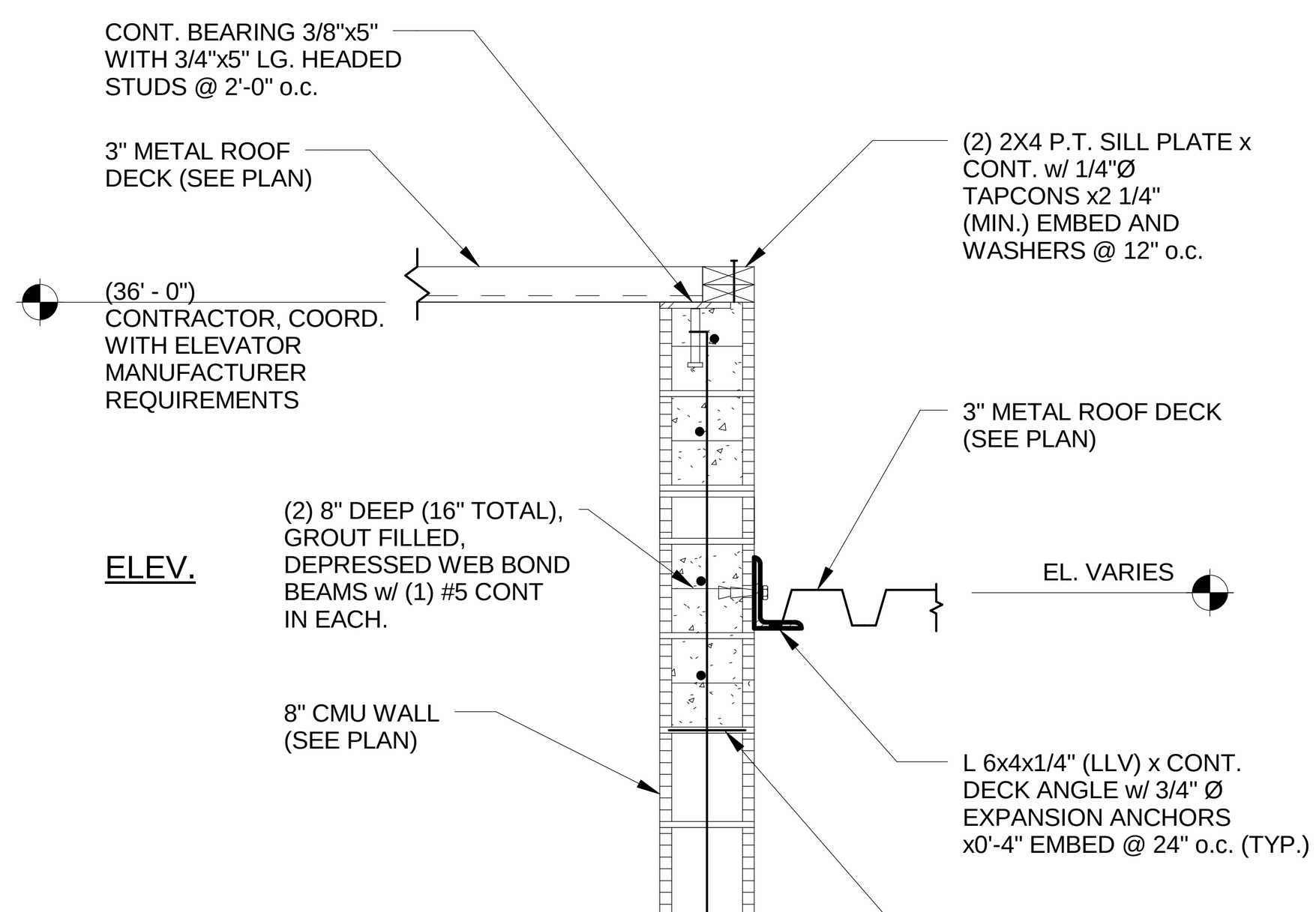


NOTE:
SEE ARCH FOR GUTTER AND FACIA NOT SHOWN

5 S4.3 ROOF DECK BEARING
3/4" = 1'-0"

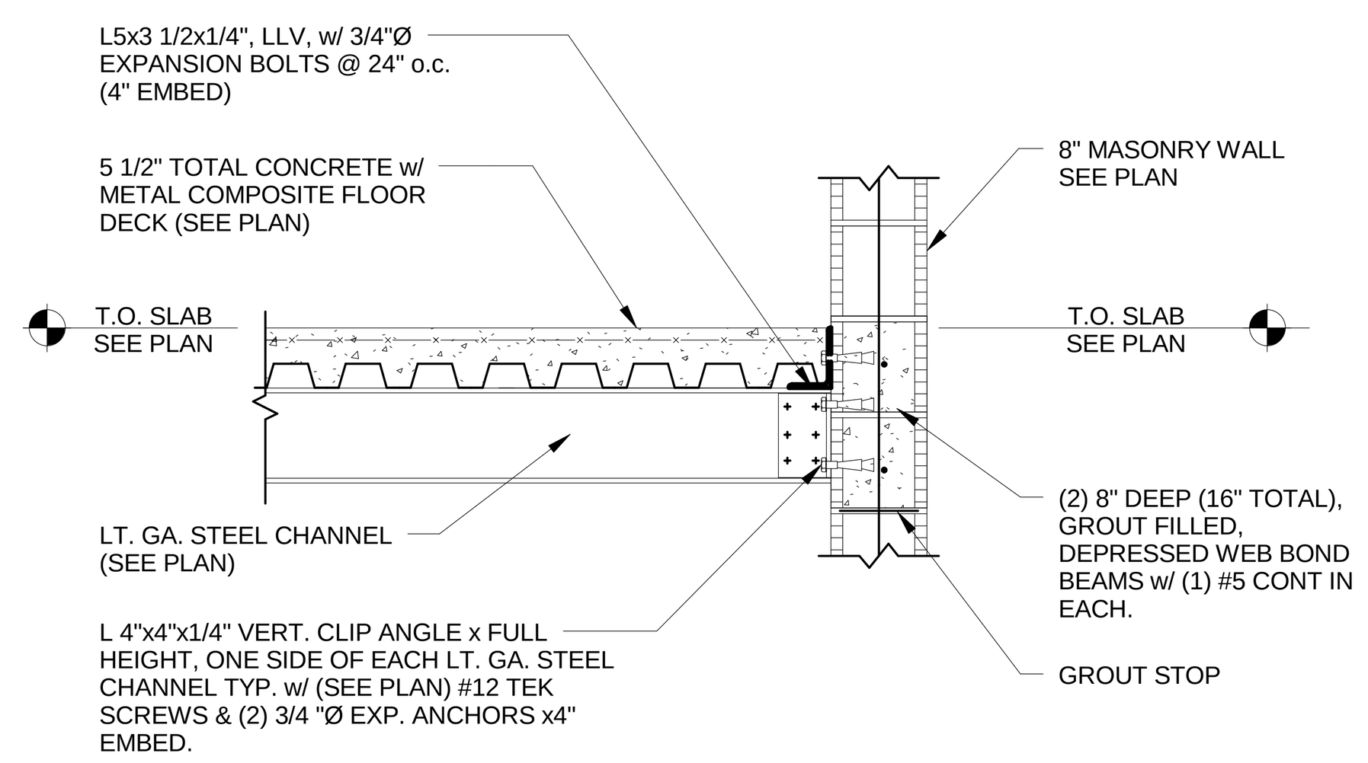


6 S4.3 FLOOR DECK BEARING
3/4" = 1'-0"

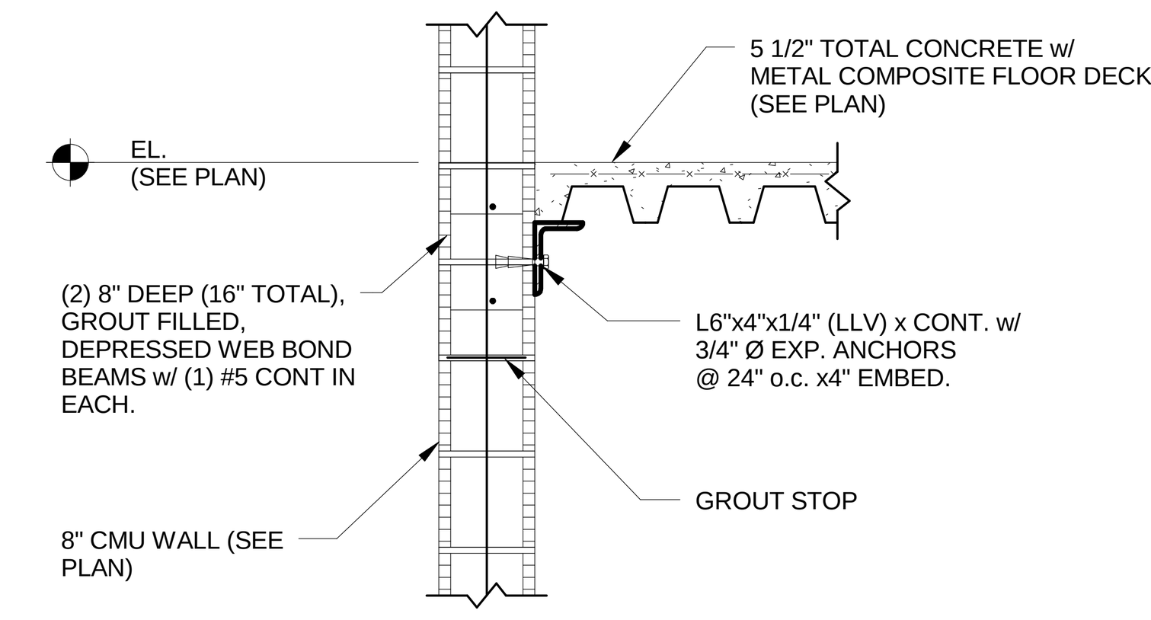


NOTE:
DECK SPAN DIRECTION CHANGES AT SIMILAR CONDITION.

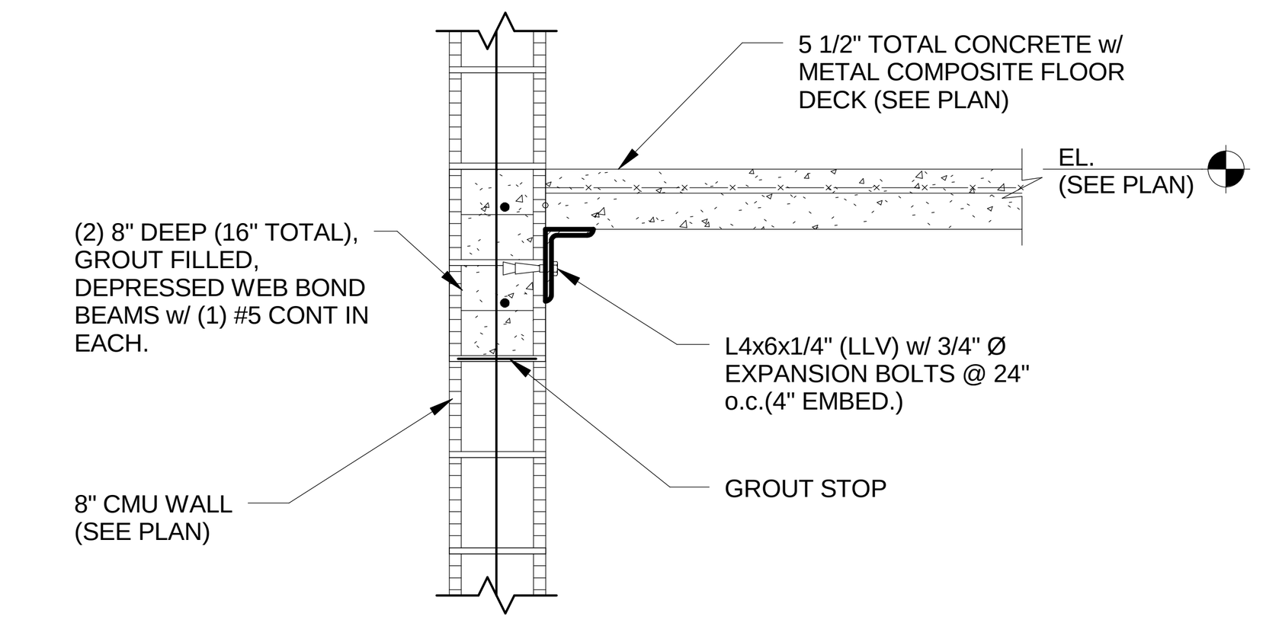
7 S4.3 DECK BEARING
3/4" = 1'-0"



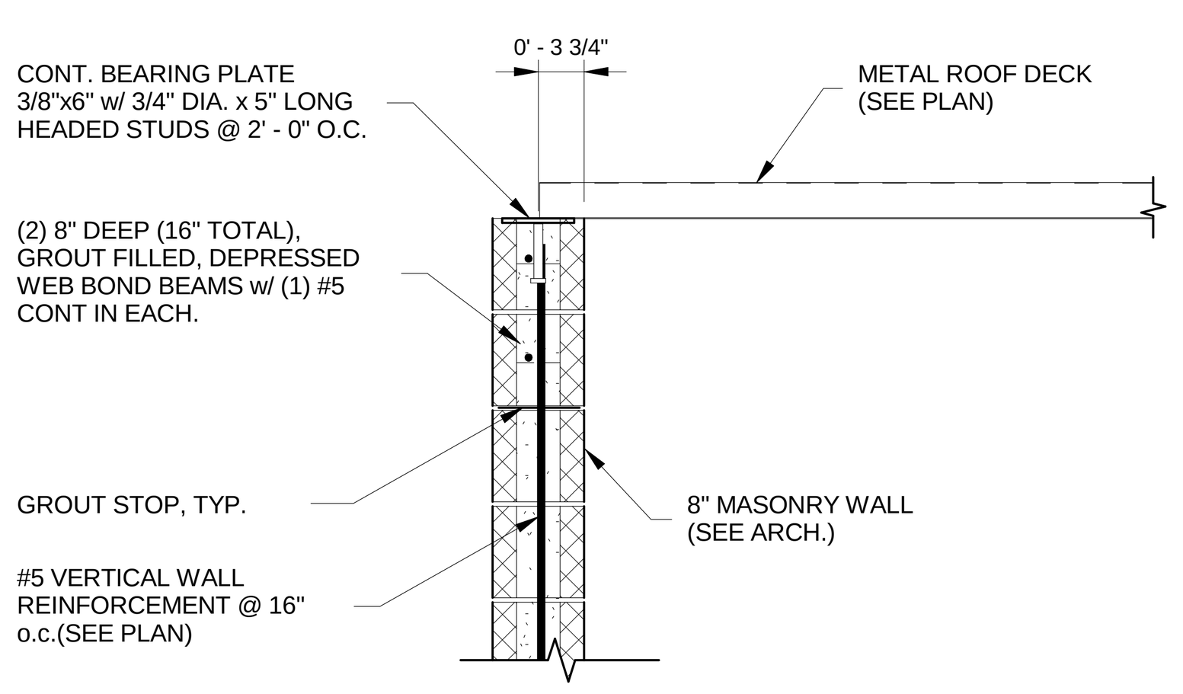
8 S4.3 L.G. CHANNEL BEAM BEARING
3/4" = 1'-0"



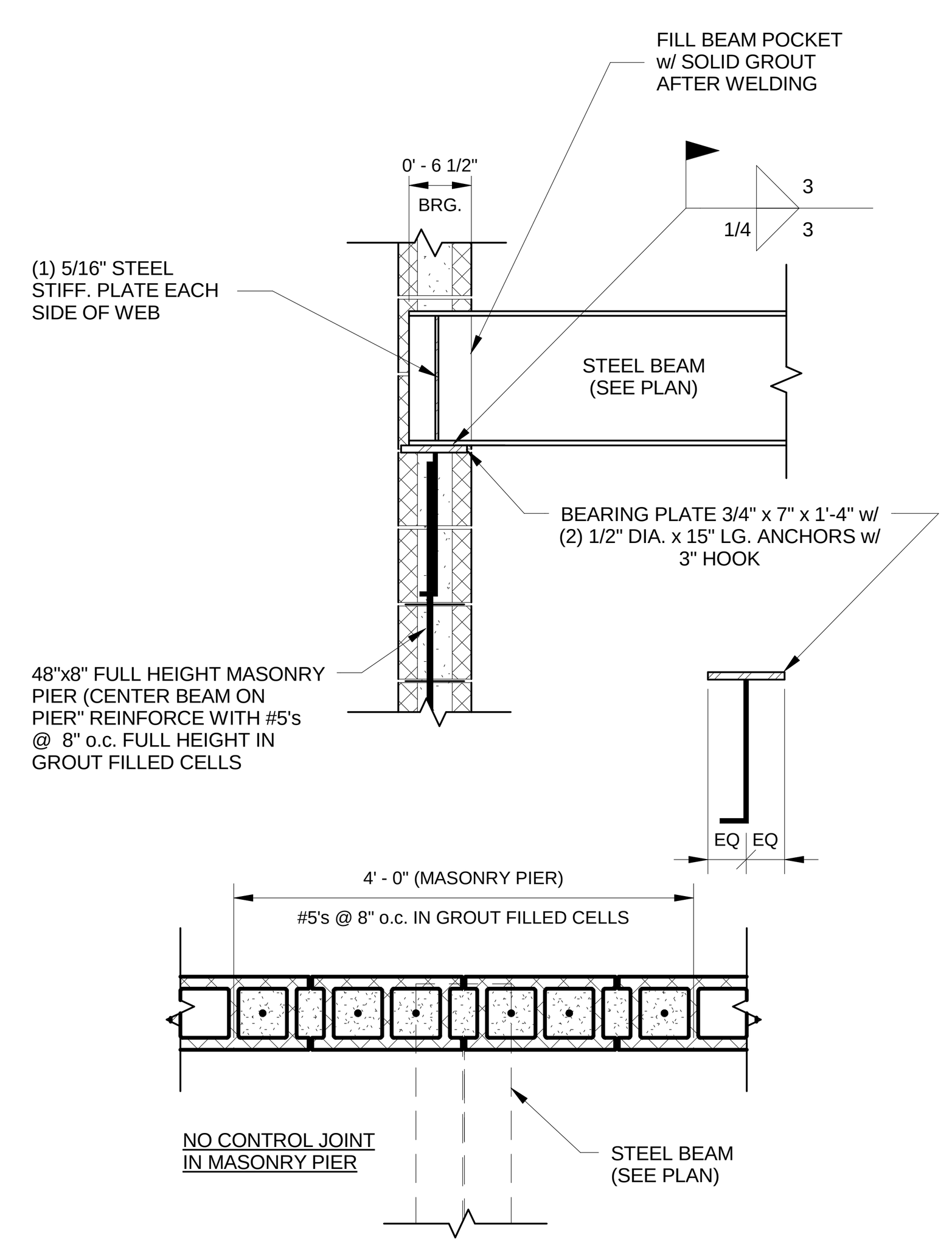
9 S4.3 FLOOR DECK BEARING
3/4" = 1'-0"



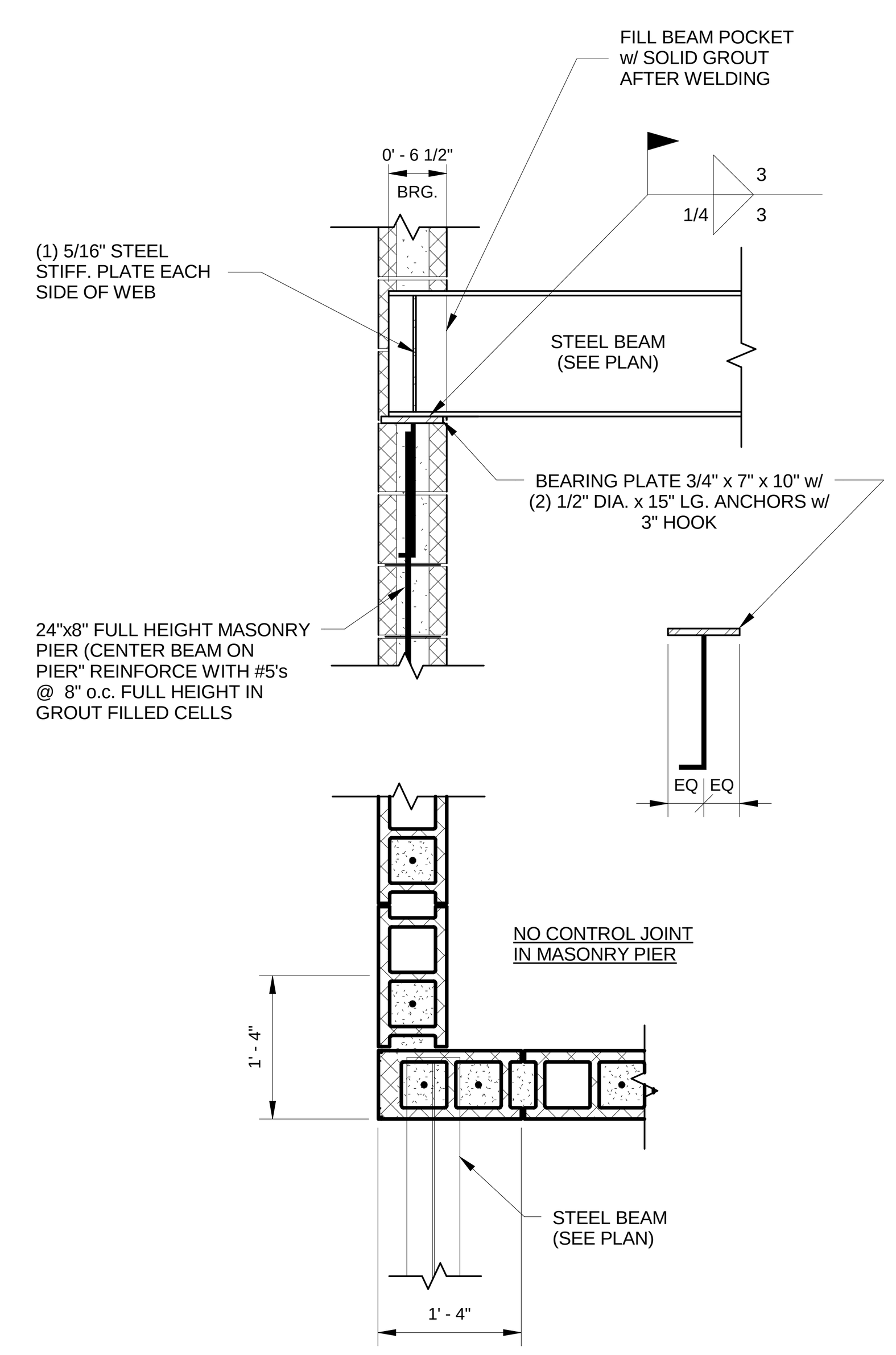
10 S4.3 FLOOR DECK BEARING
3/4" = 1'-0"



11 S4.3 DECK BEARING DETAIL
3/4" = 1'-0"



12 S4.3 BEAM BEARING ON MASONRY PIER
3/4" = 1'-0"



13 S4.3 BEAM BEARING ON MASONRY PIER
3/4" = 1'-0"

ISSUED FOR CONSTRUCTION

Keystone Structural Engineering
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P.O. Box 2654
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REVISIONS

PROJECT
924 Northside Drive Storage

ADDRESS
924 Northside Drive NW
Atlanta, GA 30318

CLIENT
Broward Management, LLC

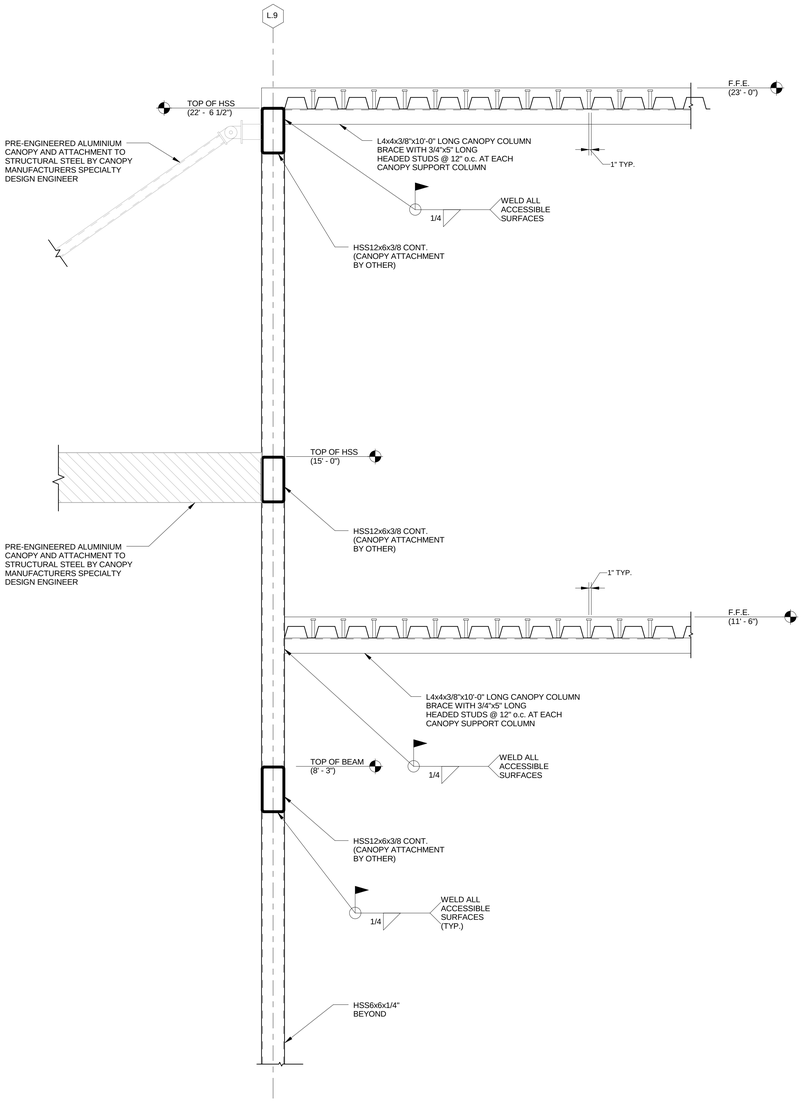
ADDRESS
6780 Roswell Rd, Suite C-200
Sandy Springs, GA 30328

SHEET TITLE
FRAMING & MASONRY DETAILS

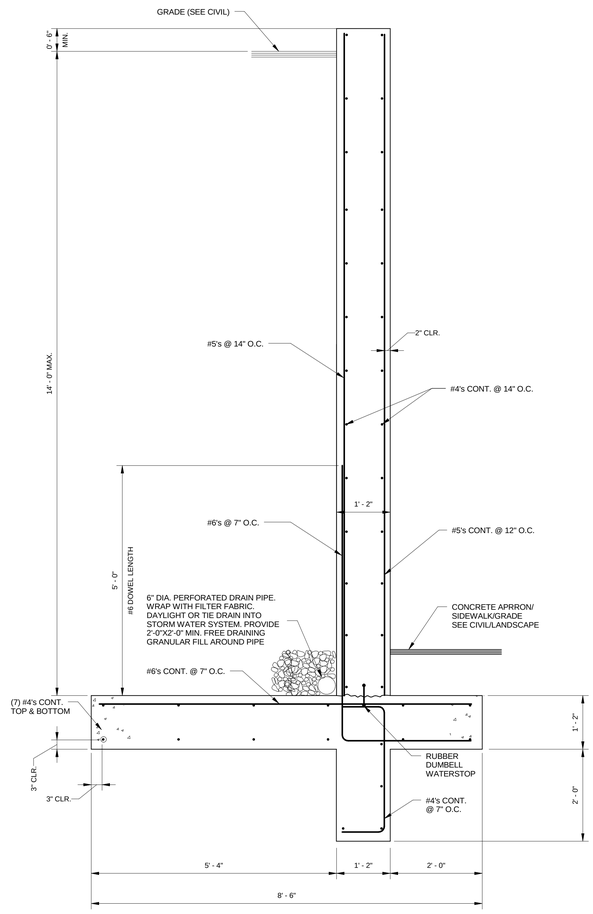
Date:
10-01-2018

PROJECT NUMBER
18-115

SHEET NUMBER
S4.3



1 S4.4 CANOPY SECTION
3/4" = 1'-0"



2 S4.4 RETAINING WALL SECTION
3/4" = 1'-0"

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GEORGIA REGISTERED PROFESSIONAL ENGINEER BRENDAN T. CROWLEY

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SHEET TITLE
FRAMING DETAILS & SECTIONS

Date:
10-01-2018

PROJECT NUMBER
18-115

SHEET NUMBER
S4.4

ISSUED FOR CONSTRUCTION

SCHEDULE OF SPECIAL INSPECTION SERVICES

MATERIAL / ACTIVITY	SERVICE	APPLICABLE TO THIS PROJECT: Y/N	EXTENT	AGENT*	DATE COMPLETED
1704.2.5 Inspection of Fabricators					
Verify fabrication/quality control procedures	In-plant review (3)	Y	Periodic	T/A	
1705.1.1 Special Cases (work unusual in nature, including but not limited to alternative materials and systems, unusual design applications, materials and systems with special manufacturer's requirements)	Submittal review, shop (3) and/or field inspection	N	N/A	N/A	
1705.2 Steel Construction					
1. Fabricator and erector documents (Verify reports and certificates as listed in AISC 360, chapter N, paragraph 3.2 for compliance with construction documents)	Submittal Review	Y	Each submittal	T/A	
2. Material verification of structural steel	Shop (3) and field inspection	Y	Periodic	T/A	
3. Embedments (Verify diameter, grade, type, length, embedment. See 1705.3 for anchors)	Field inspection	Y	Periodic	T/A	
4. Verify member locations, braces, stiffeners, and application of joint details at each connection comply with construction documents	Field inspection	Y	Periodic	T/A	
5. Structural steel welding:					
a. Inspection tasks Prior to Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-1)	Shop (3) and field inspection	Y	Observe or Perform as noted (4)	T/A	
b. Inspection tasks During Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-2)	Shop (3) and field inspection	Y	Observe (4)	T/A	
c. Inspection tasks After Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-3)	Shop (3) and field inspection	Y	Observe or Perform as noted (4)	T/A	
d. Nondestructive testing (NDT) of welded joints: *see Commentary*					
1) Complete penetration groove welds 5/16" or greater in risk category III or IV	Shop (3) or field ultrasonic testing - 100%	N	N/A	N/A	
2) Complete penetration groove welds 5/16" or greater in risk category II	Shop (3) or field ultrasonic testing - 10% of welds minimumShop (3) and field inspection	Y	Periodic	T/A	
3) Thermally cut surfaces of access holes when material t > 2"	Shop (3) or field magnetic Partical or Penetrant testing	N	N/A	N/A	
4) Welded joints subject to fatigue when required by AISC 360, Appendix 3, Table A-3.1	Shop (3) or field radiographic or Ultrasonic testing	N	N/A	N/A	
5) Fabricator's NDT reports when fabricator performs NDT	Verify Reports	Y	Periodic	T/A	
6. Structural steel bolting:	Shop (3) and field inspection				
a. Inspection tasks Prior to Bolting (Observe, or perform tasks for each bolted connection, in accordance with QA tasks listed in AISC 360, Table N5.6-1)		Y	Observe or Perform as noted (4)	T/A	
b.Inspection tasks During Bolting (Observe the QA tasks listed in AISC 360, Table N5.6-2)		Y	Observe or Perform as noted (4)	T/A	
1) Pre-tensioned and slip-critical joints		N	N/A	N/A	
a) Turn-of-nut with matching markings		N	N/A	N/A	
b) Direct tension indicator		N	N/A	N/A	
c) Twist-off type tension control bolt		N	N/A	N/A	
d) Turn-of-nut without matching markings		N	N/A	N/A	
e) Calibrated wrench		N	N/A	N/A	
2) Snug-tight joints		Y	Observe (4)	T/A	
c. Inspection tasks After Bolting (Perform tasks for each bolted connection in accordance with QA tasks listed in AISC 360, Table N5.6-3)		Y	Perform (4)	T/A	
7. Inspection of steel elements of composite construction prior to concrete placement in accordance with QA tasks listed in AISC 360, Table N6.1		N	N/A	N/A	
1705.2.2 Steel Construction Other Than Structural Steel					
1. Material verification of cold-formed steel deck:					
a. Identification markings	Field inspection	Y	Periodic	T/A	
b. Manufacturer's certified test reports	Submittal Review	Y	Each submittal	T/A	
2. Connection of cold-formed steel deck to supporting structure:	Shop (3) and field inspection				
a. Welding		Y	Periodic	T/A	
b. Other fasteners (in accordance with AISC 360,Section N6)					
1) Verify fasteners are in conformance with approved submittal		Y	Periodic	T/A	
2) Verify fastener installation is in conformance with approved submittal and manufacturer's recommendations		Y	Periodic	T/A	
3. Reinforcing steel	Shop (3) and field inspection				
a. Verification of weldability of steel other than ASTM A706		N	Continuous	N/A	
b. Reinforcing steel resisting flexural and axial forces in intermediate and special moment frames, boundary elements of special concrete structural walls and shear reinforcement		N	Continuous	N/A	
c. Shear reinforcement		N	Continuous	NA	
d. Other reinforcing steel		N	Periodic	N/A	
4. Cold-formed steel trusses spanning 60 feet or greater					
a. Verify temporary and permanent restraint/bracing are installed in accordance with the approved truss submittal package	Field inspection	N	Periodic	N/A	
1705.3 Concrete Construction					
1. Inspection of reinforcing steel installation (see 1705.2.2 for welding)	Shop (3) and field inspection	Y	Periodic	TA	
2. Inspection of prestressing steel installation	Shop (3) and field inspection	N	Periodic	N/A	
3. Inspection of anchors cast in concrete where allowable loads have been increased per section 1908.5 or where strength design is used	Shop (3) and field inspection	N	Periodic	N/A	
4. Inspection of anchors and reinforcing steel post-installed in hardened concrete: Per research reports including verification of anchor type, anchor dimensions, hole dimensions, hole cleaning procedures, anchor spacing, edge distances, concrete minimum thickness, anchor embedment and tightening torque	Field inspection	Y	Periodic or as required by the research report issued by an approved source	TA	
5. Verify use of approved design mix	Shop (3) and field inspection	Y	Periodic	TA	
6. Fresh concrete sampling, perform slump and air content tests and determine temperature of concrete	Shop (3) and field inspection	Y	Continuous	TA	
7. Inspection of concrete and shotcrete placement for proper application techniques	Shop (3) and field inspection	Y	Continuous	TA	
8. Inspection for maintenance of specified curing temperature and techniques	Shop (3) and field inspection	Y	Periodic	TA	
9. Inspection of prestressed concrete:	Shop (3) and field inspection				
a. Application of prestressing force		N	Continuous	N/A	
b. Grouting of bonded prestressing tendons in the seismic-force-resisting system		N	Continuous	N/A	
10. Erection of precast concrete members					
a. Inspect in accordance with construction documents	Field inspection	N	In accordance with construction documents	N/A	
b. Perform inspections of welding and bolting in accordance with Section 1705.2	Field inspection	N	In accordance with Section 1705.2	N/A	
11. Verification of in-situ concrete strength, prior to stressing of tendons in post tensioned concrete and prior to removal of shores and forms from beams and structural slabs	Review field testing and laboratory reports	N	Periodic	N/A	
12. Inspection of formwork for shape, lines, location and dimensions	Field inspection	Y	Periodic	TA	
13. Concrete strength testing and verification of compliance with construction documents	Field testing and review of laboratory reports	Y	Periodic	TA	
1705.4 Masonry Construction					
(A) Level A, B and C Quality Assurance:					
1. Verify compliance with approved submittals	Field inspection	Y	Periodic	TA	
(B) Level B Quality Assurance:					
1. Verification of f'm and f'AAC prior to construction	Testing by unit strength method or prism test method	Y	Periodic	EOR	
(C) Level C Quality Assurance:					
1. Verification of f'm and f'AAC prior to construction and for every 5,000 SF during construction	Testing by unit strength method or prism test method	Y	Periodic	TA	
2. Verification of proportions of materials in premixed or preblended mortar, prestressing grout, and grout other than self-consolidating grout, as delivered to the project site	Field inspection	Y	Periodic	TA	
3. Verify placement of masonry units	Field inspection	Y	Periodic	TA	
(D) Levels B and C Quality Assurance:					
1. Verification of Slump Flow and Visual Stability Index (VSI) of self-consolidating grout as delivered to the project	Field testing	Y	Continuous	TA	
2. Verify compliance with approved submittals	Field inspection	Y	Periodic	TA	
3. Verify proportions of site-mixed mortar, grout and prestressing grout for bonded tendons	Field inspection	Y	Periodic	TA	
4. Verify grade, type, and size of reinforcement and anchor bolts, and prestressing tendons and anchorages	Field inspection	Y	Periodic	TA	
5. Verify construction of mortar joints	Field inspection	Y	Periodic	TA	
6. Verify placement of reinforcement, connectors, and prestressing tendons and anchorages	Field inspection	Y	Level B - Periodic	TA	
			Level C - Continuous		
7. Verify grout space prior to grouting	Field inspection		Periodic		
		Y	Continuous	TA	
8. Verify placement of grout and prestressing grout for bonded tendons	Field inspection	Y	N/A	N/A	
9. Verify size and location of structural masonry elements	Field inspection	Y	Periodic	TA	
10. Verify type, size, and location of anchors, including details of anchorage of masonry to structural members, frames, or other construction.	Field inspection	Y	Level B - Periodic	TA	
			Level C - Continuous		
11. Verify welding of reinforcement (see 1705.2.2)	Field inspection	Y	Continuous	N/A	
12. Verify preparation, construction, and protection of masonry during cold weather (temperature below 40-F) or hot weather (temperature above 90-F)	Field inspection	Y	Periodic	TA	
13. Verify application and measurement of prestressing force	Field inspection	N	Continuous	N/A	
14. Verify placement of AAC masonry units and construction of thin-bed mortar joints (first 5000 SF of AAC masonry)	Field inspection	N	Continuous	N/A	
15. Verify placement of AAC masonry units and construction of thin-bed mortar joints (after the first 5000 SF of AAC masonry)	Field inspection	N	Level B - Periodic	N/A	
		N	Level C - Continuous	N/A	
16. Verify properties of thin-bed mortar for AAC masonry (first 5000 SF of AAC masonry)	Field inspection	N	Continuous	N/A	
17. Verify properties of thin-bed mortar forAAC masonry (after the first 5000 SF of AAC masonry)	Field inspection	N	Level B - Periodic	N/A	
		N	Level C - Continuous	N/A	
18. Prepare grout and mortar specimens	Field testing	Y	Level B - Periodic	TA	
			Level C - Continuous		
19. Observe preparation of prisms	Field inspection		Level B - Periodic		
		Y	Level C - Continuous	TA	
1705.5 Wood Construction		N			
1. Inspection of the fabrication process of wood structural elements and assemblies in accordance with Section 1704.2.5	In-plant review (3)	N	Periodic	N/A	
2. For high-load diaphragms, verify grade and thickness of structural panel sheathing agree with approved building plans	Field inspection	N	Periodic	N/A	
3. For high-load diaphragms, verify nominal size of framing members at adjoining panel edges, nail or staple diameter and length, number of fastener lines, and that spacing between fasteners in each line and at edge margins agree with approved building plans	Field inspection	N	Periodic	N/A	
4. Metal-plate-connected wood trusses spanning 60 feet or greater: verify temporary and permanent restraint/bracing are installed in accordance with the approved truss submittal package	Field inspection	N	Periodic	N/A	
1705.6 Soils					
1. Verify materials below shallow foundations are adequate to achieve the design bearing capacity.	Field inspection	Y	Periodic	TA	
2. Verify excavations are extended to proper depth and have reached proper material.	Field inspection	Y	Periodic	TA	
3. Perform classification and testing of controlled fill materials.	Field inspection	Y	Periodic	TA	
4. Verify use of proper materials, densities, and lift thicknesses during placement and compaction of controlled fill	Field inspection	Y	Continuous	TA	
5. Prior to placement of controlled fill, observe subgrade and verify that site has been prepared properly	Field inspection	Y	Periodic	TA	
1705.7 Driven Deep Foundations		N			
1. Verify element materials, sizes and lengths comply with requirements	Field inspection	N	Continuous	N/A	
2. Determine capacities of test elements and conduct additional load tests, as required	Field inspection	N	Continuous	N/A	
3. Observe driving operations and maintain complete and accurate records for each element	Field inspection	N	Continuous	N/A	
4. Verify placement locations and plumbness, confirm type and size of hammer, record number of blows per foot of penetration, determine required penetrations to achieve design capacity, record tip and butt elevations and document any damage to foundation element	Field inspection	N	Continuous	N/A	
5. For steel elements, perform additional inspections per Section 1705.2	See Section 1705.2	N	See Section 1705.2	N/A	
6. For concrete elements and concrete-filled elements, perform additional inspections per Section 1705.3	See Section 1705.3	N	See Section 1705.3	N/A	
7. For specialty elements, perform additional inspections as determined by the registered design professional in responsible charge	Field inspection	N	In accordance with construction documents	N/A	
8. Perform additional inspections and tests in accordance with the construction documents	Field Inspection and testing	N	In accordance with construction documents	N/A	
1705.8 Cast-in-Place Deep Foundations		N			
1.Observe drilling operations and maintain complete and accurate records for each element	Field inspection	N	Continuous	N/A	
2. Verify placement locations and plumbness, confirm element diameters, bell diameters (if applicable), lengths, embedment into bedrock (if applicable) and adequate end-bearing strata capacity. Record concrete or grout volumes	Field inspection	N	Continuous	N/A	
3. For concrete elements, perform additional inspections in accordance with Section 1705.3	See Section 1705.3	N	See Section 1705.3	N/A	
4. Perform additional inspections and tests in accordance with the construction documents	Field Inspection and testing	N	In accordance with construction documents	N/A	
1705.9 Helical Pile Foundations		Y			
1. Verify installation equipment, pile dimensions, tip elevations, final depth, final installation torque and other data as required.	Field inspection	Y	Continuous	N/A	
2. Perform additional inspections and tests in accordance with the construction documents	Field Inspection and testing	Y	In accordance with construction documents	N/A	
1705.10.1 Structural Wood Special Inspections For Wind Resistance		N			
1. Inspection of field gluing operations of elements of the main windforce-resisting system	Field inspection	N	Continuous	N/A	
2. Inspection of nailing, bolting, anchoring and other fastening of components within the main windforce-resisting system	Shop (3) and field inspection	N	Periodic	N/A	
1705.10.2 Cold-formed Steel Special Inspections For Wind Resistance		N			
1.Inspection during welding operations of elements of the main windforce-resisting system	Shop (3) and field inspection	N	Periodic	N/A	
2.Inspections for screw attachment, bolting, anchoring and other fastening of components within the main windforce-resisting system	Shop (3) and field inspection	N	Periodic	N/A	
1705.10.3 Wind-resisting Components					
1. Roof cladding	Shop (3) and field inspection	N	Periodic	TA	
2. Wall cladding	Shop (3) and field inspection	N	Periodic	TA	
1705.11.1 Structural Steel Special Inspections for Seismic Resistance					
Inspection of structural steel in accordance with AISC 341	Shop (3) and field inspection	Y	In accordance with AISC 341	N/A	
1705.11.2 Structural Wood Special Inspections for Seismic Resistance					
1. Inspection of field gluing operations of elements of the seismic-force resisting system	Field inspection	N	Continuous	N/A	
2. Inspection of nailing, bolting, anchoring and other fastening of components within the seismic-force-resisting system	Shop (3) and field inspection	N	Periodic	N/A	
1705.11.3 Cold-formed Steel Light-Frame Construction Special Inspections for Seismic Resistance					
1. Inspection during welding operations of elements of the seismic-force-resisting system	Shop (3) and field inspection	Y	Periodic	N/A	
2. Inspections for screw attachment, bolting, anchoring and other fastening of components within the seismic-force-resisting system	Shop (3) and field inspection	Y	Periodic	N/A	
1705.11.4 Designated Seismic Systems Verification					
Inspect and verify that that the component label, anchorage or mounting conforms to the certificate of compliance in accordance with Section 1705.12.3	Field inspection	N	Periodic	N/A	
1705.11.5 Architectural Components Special Inspections for Seismic Resistance					
1. Inspection during the erection and fastening of exterior cladding and interior and exterior veneer	Field inspection	N	Periodic	N/A	
2. Inspection during the erection and fastening of interior and exterior nonbearing walls	Field inspection	N	Periodic	N/A	
3. Inspection during anchorage of access floors	Field inspection	N	Periodic	N/A	
1705.11.6 Mechanical and Electrical Components Special Inspections for Seismic Resistance					
1. Inspection during the anchorage of electrical equipment for emergency or standby power systems	Field inspection	N	Periodic	TA	
2. Inspection during the anchorage of other electrical equipment	Field inspection	N	Periodic	TA	
3. Inspection during installation and anchorage of piping systems designed to carry hazardous materials, and their associated mechanical units	Field inspection	N	Periodic	TA	
4. Inspection during the installation and anchorage of HVAC ductwork that will contain hazardous materials	Field inspection	N	Periodic	N/A	
5. Inspection during the installation and anchorage of vibration isolation systems	Field inspection	N	Periodic	N/A	
1705.11.7 Storage Racks Special Inspections for Seismic Resistance					
Inspection during the anchorage of storage racks 8 feet or greater in height	Field inspection	N	Periodic	TA	
1705.11.8 Seismic Isolation Systems					
Inspection during the fabrication and installation of isolator units and energy dissipation devices used as part of the seismic isolation system	Shop and field inspection	N	Periodic	N/A	
1705.12.1 Concrete Reinforcement Testing and Qualification for Seismic Resistance					
1. Review certified mill test reports for each shipment of reinforcement used to resist earthquake-induced flexural and axial forces in reinforced concrete special moment frames, special structural walls, and coupling beams connecting special structural walls	Review certified mill test reports	N	Each shipment	N/A	
2. Verify reinforcement weldability of ASTM A615 reinforcement used to resist earthquake-induced flexural and axial forces in reinforced concrete special moment frames, special structural walls, and coupling beams connecting special structural walls	Review test reports	N	Each shipment	N/A	
1705.12.2 Structural Steel Testing and Qualification for Seismic Resistance		N			
Test in accordance with the quality assurance requirements of AISC 341		N	Per AISC 341	N/A	
1705.12.3 Seismic Certification of Nonstructural Components		N			
Review certificate of compliance for designated seismic system components.	Certificate of compliance review	N	Each submittal	N/A	
1705.12.4 Seismic Isolation Systems					
Test seismic isolation system in accordance with ASCE 7 Section 17.8	Prototype testing	N	Per ASCE 7	N/A	
1705.13 Sprayed Fire-resistant Materials					
1. Verify surface condition preparation of structural members	Field inspection	N	Periodic	N/A	
2. Verify application of sprayed fire-resistant materials	Field inspection	N	Periodic	N/A	
3. Verify average thickness of sprayed fire-resistant materials applied to structural members	Field inspection	N	Periodic	N/A	
4. Verify density of the sprayed fire-resistant material complies with approved fire-resistant design	Field Inspection and testing	N	Per IBC Section 1705.13.5	N/A	
5. Verify the cohesive/adhesive bond strength of the cured sprayed fire-resistant material	Field Inspection and testing	N	Per IBC Section 105.13.6	N/A	
1705.14 Mastic and Intumescent Fire-Resistant Coatings		N			
Inspect mastic and intumescent fire-resistant coatings applied to structural elements and decks	Field inspection	N	Periodic	N/A	
1705.15 Exterior Insulation and Finish Systems (EIFS)					
1. Verify materials, details and installations are per the approved construction documents	Field inspection	Y	Periodic	TA	
2. Inspection of water-resistive barrier over sheathing substrate	Field inspection	Y	Periodic	TA	
1705.16 Fire-Resistant Penetrations and Joints		N			
1. Inspect penetration firestop	Field testing	N	Per ASTM E2174	N/A	
2. Inspect fire-resistant joint systems	Field testing	N	Per ASTM E2174	N/A	
1705.17 Smoke Control Systems		N			
1. Leakage testing and recording of device locations prior to concealment	Field testing	N	Periodic	N/A	
2. Prior to occupancy and after sufficient completion, pressure difference testing, flow measurements, and detection and control verification	Field testing	N	Periodic	N/A	

*** INSPECTION AGENTS**

FIRM	ADDRESS	TELEPHONE NO.
1.		
2.		
3.		
4.		

NOTES:
1. *The inspection and testing agent(s) shall be engaged by the Owner or the Owner's Agent, and not by the Contractor or Subcontractor whose work is to be inspected or tested. Any conflict of interest must be disclosed to the Building Official prior to commencing work. The qualifications of the Special Inspector(s) and/or testing agencies may be subject to the approval of the Building Official and/or the Design Professional.*
2. *The list of Special Inspectors may be submitted as a separate document, if noted so above.*
3. *Special Insepctions as required by Section 1704.2.5 are not required where the fabricator is approved in accordance with IBC Section 1704.2.5.2*
4. *Observe on a random basis, operations need not be delayed pending these inspections. Perform these tasks for each welded joint, bolted connection, or steel element.*
5. *NDT of welds completed in an approved fabricator's shop may be performed by that fabricator when approved by the AHJ. Refer to AISC 360, N7.*

Are Requirements for Seismic Resistance included in the Statement of Special Inspections? Yes
Are Requirements for Wind Resistance included in the Statement of Special Inspections? No

Abbreviations:
TA = Testing Agency
EOR = Engineer of Record
AOR = Architect of Record
N/A = Not Applicable

ISSUED FOR CONSTRUCTION

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GEORGIA REGISTERED PROFESSIONAL ENGINEER No. 30737 BRENDAN F. CROWLEY

REVISIONS

PROJECT
924 Northside Drive Storage
ADDRESS
924 Northside Drive NW
Atlanta, GA 30318

CLIENT
Broward Management, LLC
ADDRESS
6780 Roswell Rd, Suite C-200
Sandy Springs, GA 30328

SHEET TITLE
SCHEDULE OF SPECIAL INSPECTIONS

Date:
10-01-2018

PROJECT NUMBER
18-115

SHEET NUMBER
S5.1